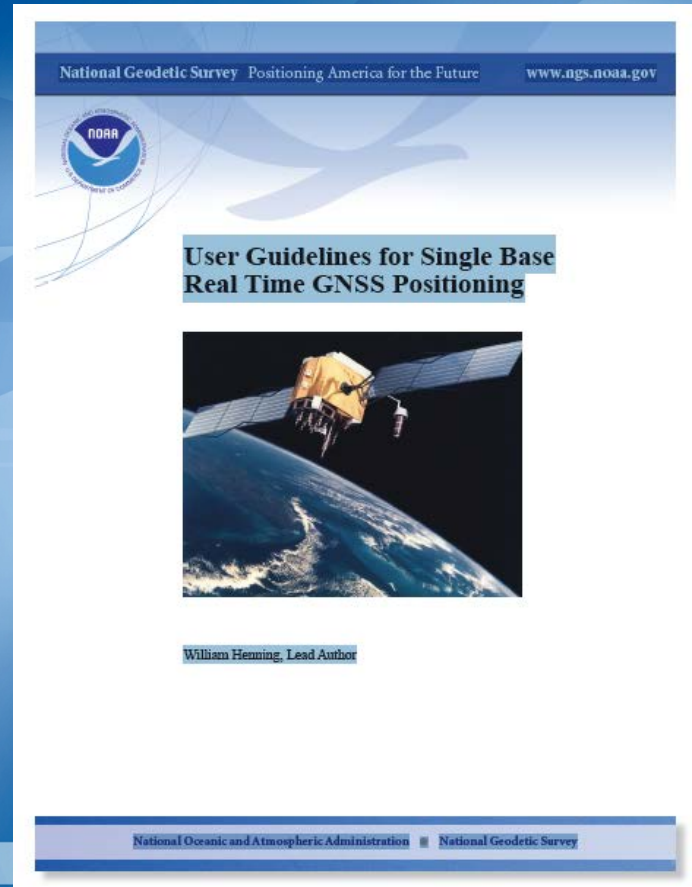
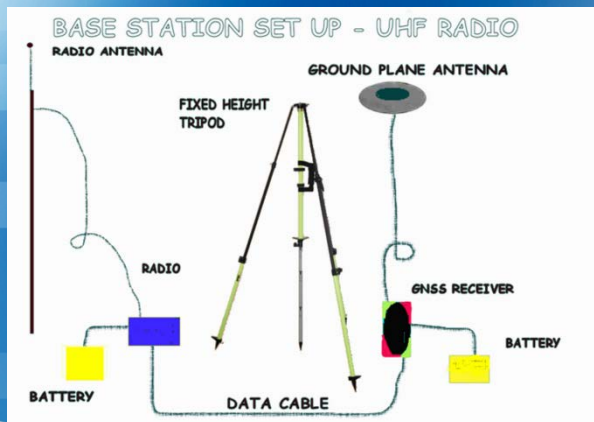


NGS WEBINAR NOVEMBER 9, 2011

REAL TIME GNSS POSITIONING BEST METHODS FOR THE FIELD



THE IMPORTANCE OF INTELLIGENT RT FIELD WORK

- Multipath
- Position Dilution of Precision (PDOP)
- Baseline Root Mean Square (RMS)
- Number of satellites
- Elevation mask (or cut-off angle)
- Base accuracy- datum level, local level
- Base security
- Redundancy, redundancy, redundancy
- Part(s) Per Million Error (ppm) – iono, tropo models, orbit errors
- Space weather- sunspot numbers, solar maximum
- Geoid quality
- Site calibrations (a.k.a. Localizations)
- Bubble adjustment
- Latency, update rate
- Fixed and float solutions
- Accuracy versus Precision
- Signal to Noise Ratio (S/N or C/N0)
- Carrier phase solution
- Code phase solution
- VHF/UHF radio communication
- CDMA/SIM/Cellular TCP/IP communication
- WGS 84 versus NAD 83, or other local datums
- GPS, GLONASS, Galileo, Compass Constellations

**THE CONTROL IS AT
THE POLE!**



WORKSHOP PLAN

INTRODUCTION-CHANGE AS A WAY OF LIFE



BACKGROUND- DATUMS, RTN vs SINGLE BASE



HOW RT GNSS POSITIONING (RT) WORKS- BEYOND THE BLACK BOX



GEOMETRICAL & DISPERSIVE EFFECTS ON THE GNSS SIGNAL



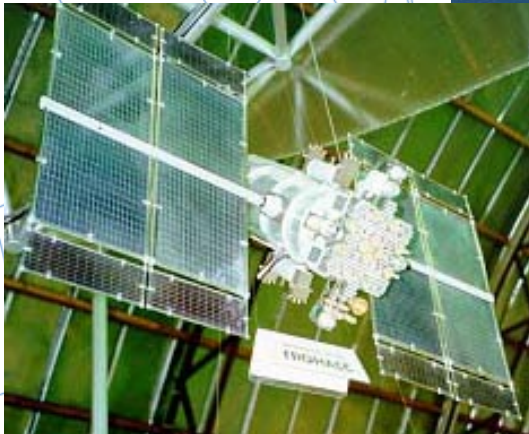
BEST METHODS FOR GNSS REAL TIME POSITIONING- A FIELD GUIDE



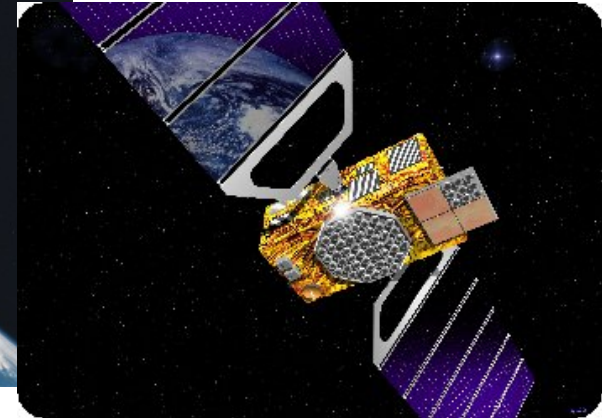
GNSS POSITIONING TECHNOLOGY- WHERE WE STAND NOW AND WHERE WE ARE HEADED



GPS



GLONASS



GALILEO

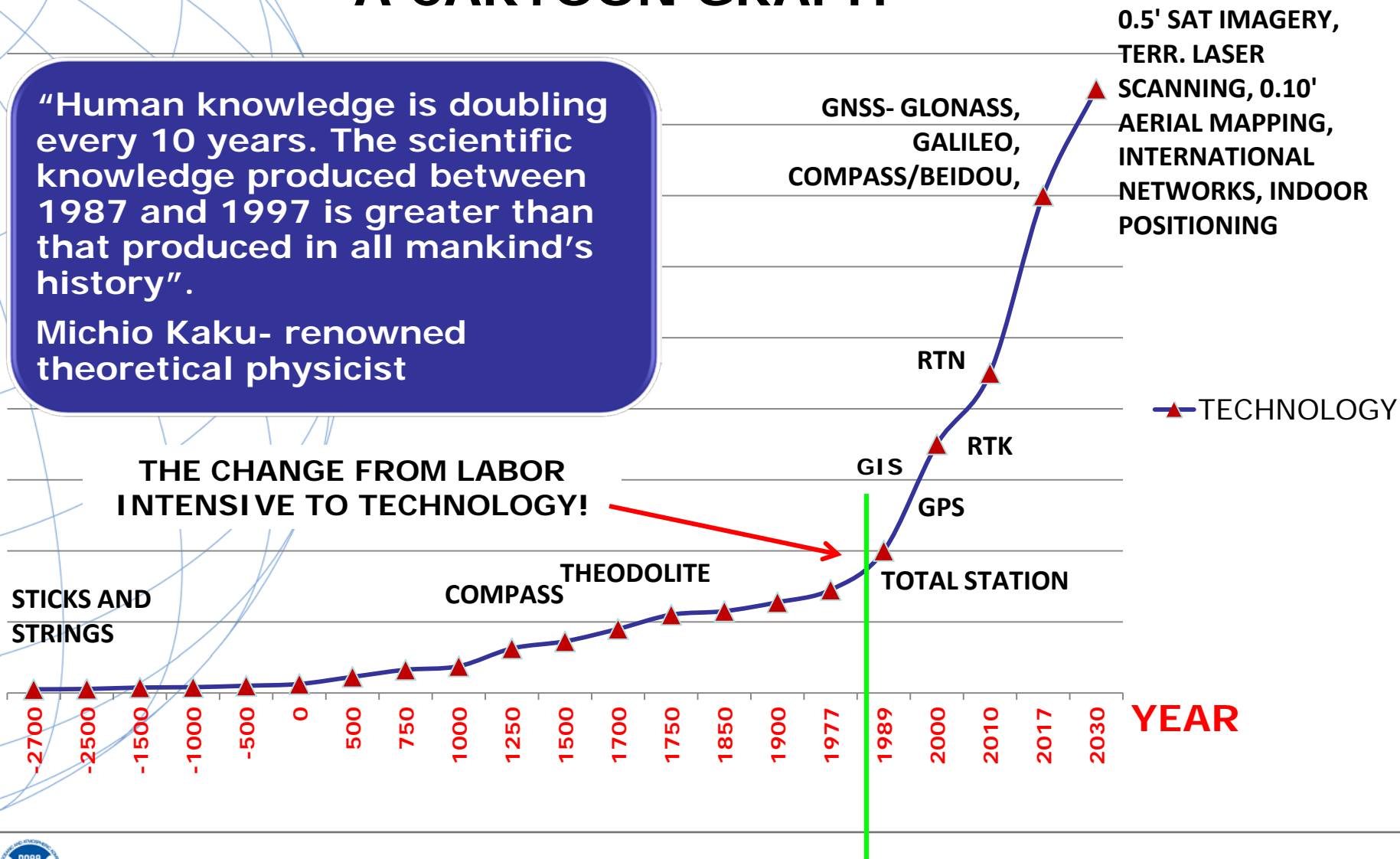


COMPASS (BEIDOU-2)

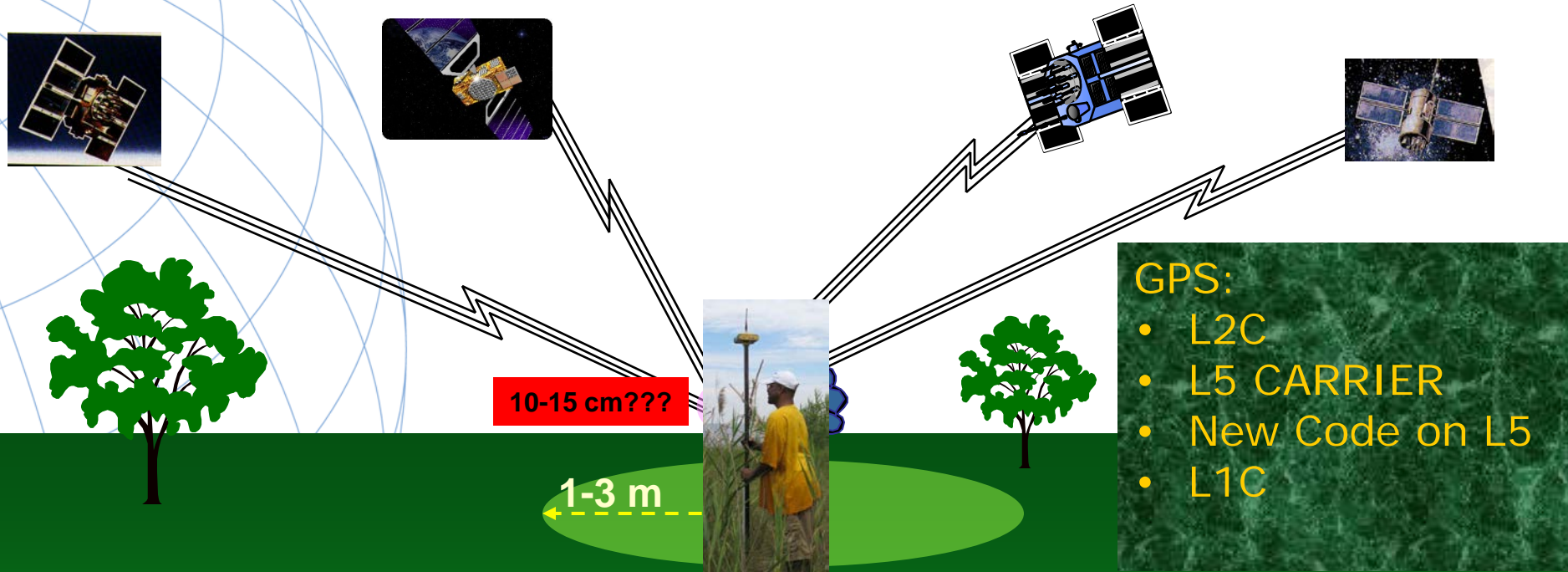
POSITIONING TECHNOLOGY- A CARTOON GRAPH

"Human knowledge is doubling every 10 years. The scientific knowledge produced between 1987 and 1997 is greater than that produced in all mankind's history".

Michio Kaku- renowned theoretical physicist



CHANGES IN GNSS



GLONASS- FULL OPERATIONAL CAPABILITY 2011

EUROPEAN UNION - GALILEO

CHINA – COMPASS/BEIDOU

= 115 SATELLITES?

REGIONAL:
(JAPAN- QZSS FIRST LAUNCH 2010)
(INDIA – GAGAN)

**BETTER
RESISTANCE TO
INTERFERENCE**

**FASTER
AMBIGUITY
RESOLUTION**

**AUGMENTED
CODE
APPLICATIONS**

GALILEO- OFF THE GROUND

OCTOBER 21, 2011



**2 IOV SATELLITES, EUROPEAN SPACEPORT, FRENCH
GUIANA, RUSSIAN SOYEZ ROCKET (SPUTNIK, YURI
GREGARIN)**

THE USE OF RTK- A CONFLUENCE OF TECHNOLOGY



- **INTERNET DATA VIA CELL TECHNOLOGY**
- **SOFTWARE/FIRMWARE ALGORITHMS**
- **GNSS HARDWARE**
- **SATELLITE CONSTELLATIONS**
- **SATELLITE CODES/FREQUENCIES**

TECHNOLOGY

SURVEYING

GEODESY

ENGINEERING

REMOTE SENSING

MAPPING

CM LEVEL
PRECISION/ACCURACY

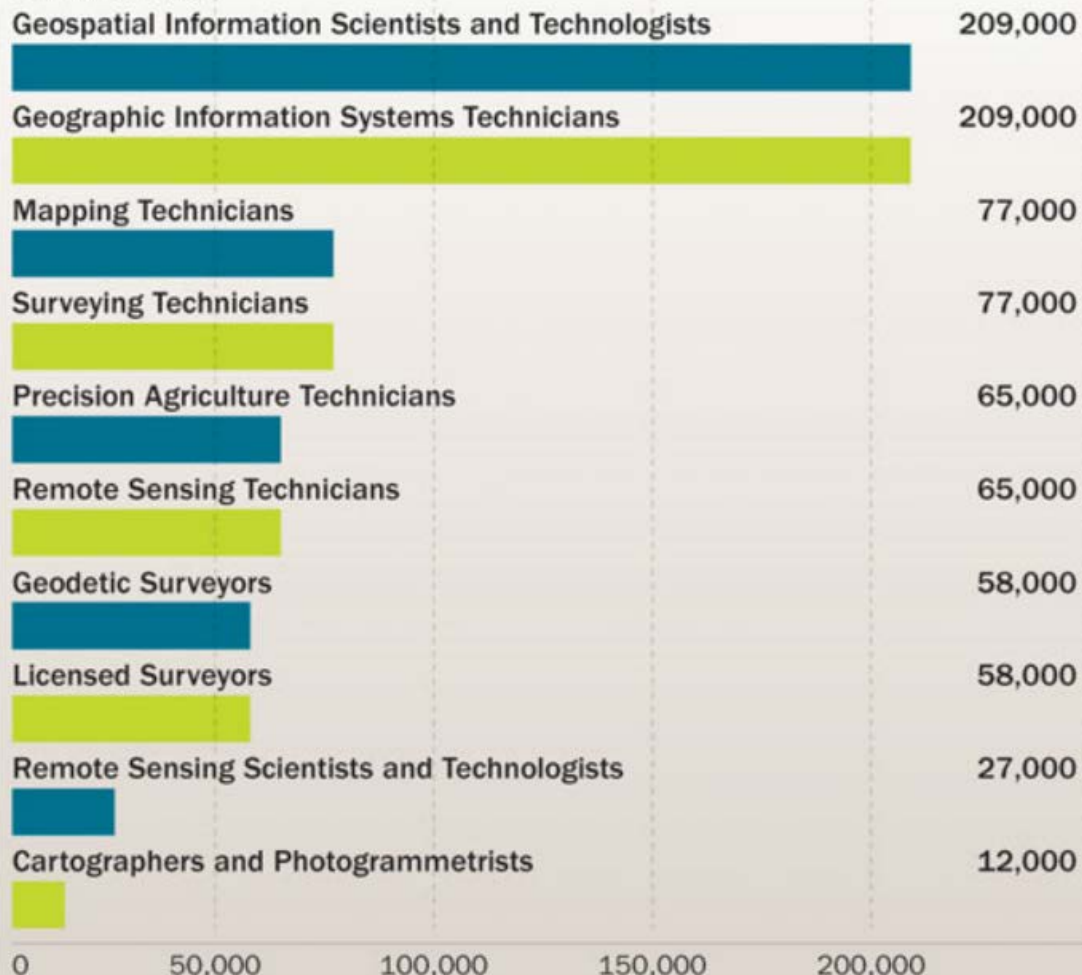
AROUND 2020
AUTONOMOUS GNSS
POSITIONING MAY BE
BETTER THAN 2-DM

GEOSPATIAL PROFESSIONALS

Graphic:
Jeffery N. Lucas, PLS, Esq.
POB Magazine
On-Line Article
September 28, 2011
www.POBonline.com

The Geospatial Occupations 2008 Labor Department Statistics

OCCUPATION:



TOTALS

857,000



MULTI-YEAR CORS SOLUTION (MYCS)

= **NAD 83 (2011)**:

- BETTER ORBITS <http://geodesy.noaa.gov/CORS/coords.shtml>
- MORE TRUE VELOCITIES KNOWN (1200 STATIONS +/- WITH >2.5 YEARS OF DATA-OTHERS MODELED WITH HTDP)
- **ABSOLUTE** ANTENNA CALIBRATIONS
- BROUGHT TO A MORE CURRENT EPOCH (**2002 to 2010**)
- IMPLEMENTED **SEPT 2011**

PASSIVE CONTROL ADJUSTMENT TO MYCS:

- ORIGINAL OBSERVATIONS!!
- NO VELOCITIES KNOWN (MODELED ONLY)
- NO RIGOROUS ADJUSTMENT
- 5000 GPS PROJECTS SIMULTANEOUSLY VIA "NETSTAT" TO 95% CONFIDENCE
- IMPLEMENTED **EARLY CALENDAR YEAR 2012 +/-**
- WILL HAVE NETWORK & **LOCAL** ACCURACIES

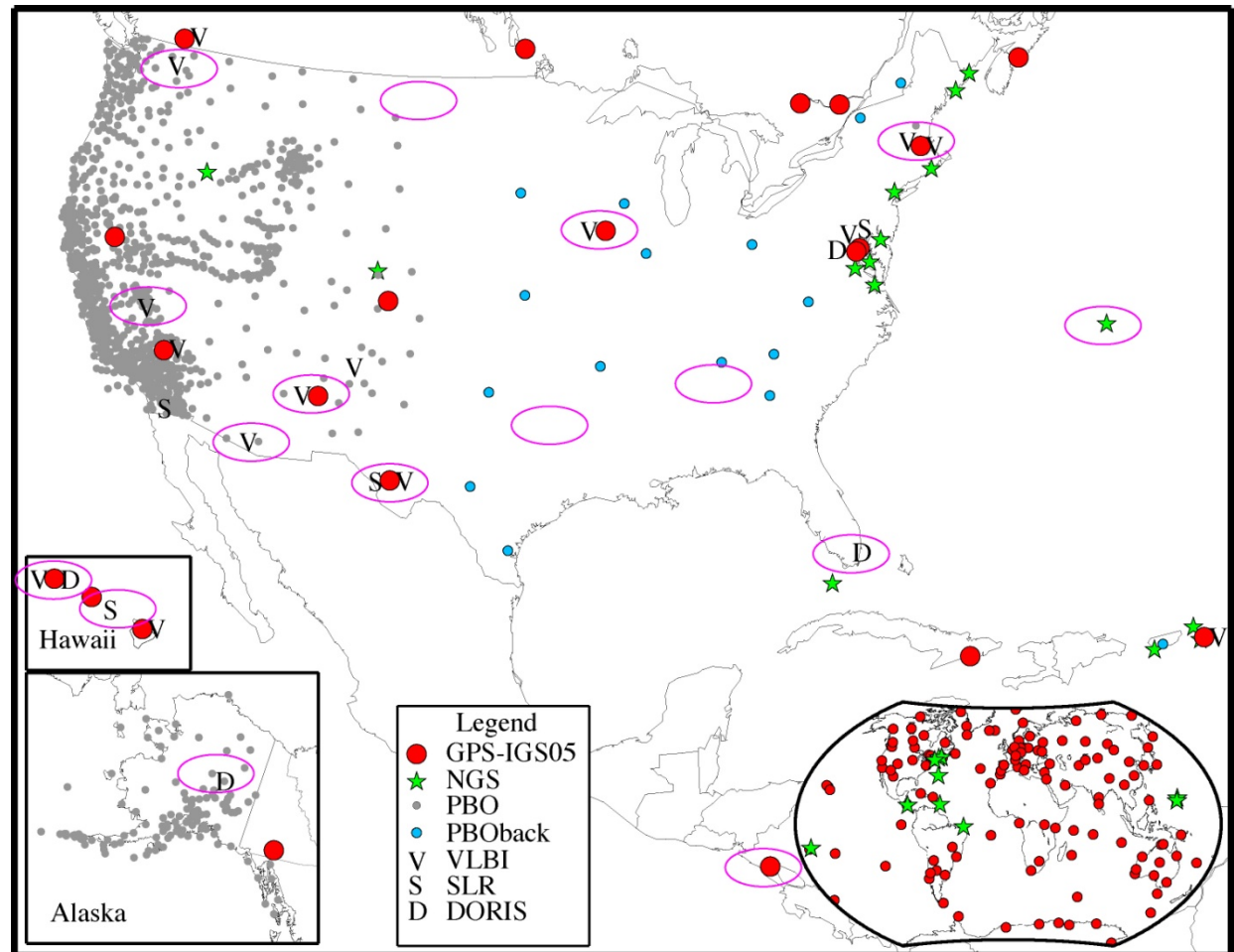


NEW GEOMETRICAL DATUM 2022 +/-

FOUNDATION CORS

FEDERALLY
OWNED/OPERATED
BASE NETWORK =
ULTIMATE TRUTH

- Link to ITRF at sites co-located with VLBI, SLR, DORIS, then fill geographic gaps



Possible sites = magenta ovals

NEW NATIONAL VERTICAL DATUM 2022?

- A PURELY GRAVIMETRIC SURFACE
- BASED ON A HIGH RESOLUTION, 1 CM GEOID FROM GRAV-D PROGRAM
- ACCURATE TO 2 CM (ALLOWING FOR GNSS ERROR)
- ACCESSABILITY: BROUGHT TO A PROJECT SITE VIA ACTIVE REFERENCE STATIONS (NATIONAL CORS), DENSIFIED TO PROJECT ACCURACY NEEDS. (ALTERNATIVE: USE BMs PREVIOUSLY TIED TO THE DATUM-CAVEAT EMPTOR)

WHAT ABOUT THE PASSIVE MARKS?

- **SECONDARY ACCESS** (USE AT YOUR OWN RISK)
 - PASSIVE MARKS THAT HAVE BEEN TIED TO THE NEW VERTICAL DATUM
 - NGS WILL PROVIDE A “DATA SHARING” SERVICE FOR THESE POINTS, BUT THEIR ACCURACY (DUE TO EITHER THE QUALITY OF THE SURVEY OR THE AGE OF THE DATA) WILL NOT BE A RESPONSIBILITY OF NGS
 - A CONVERSION WILL BE PROVIDED BETWEEN NAVD 88 AND THE NEW DATUM WHERE RECENT GNSS ELLIPSOID HEIGHTS EXIST TO PROVIDE MODERN HEIGHTS IN THE NEW DATUM

2022 NEW PROJECT CONTROL – ACCESS TO NSRS

RTN ALIGNED TO
CORS AT 1 CM IN
EACH
COMPONENT
(X,Y,Z) & 2 CM
ORTHOMETRIC

NEW PROJECT
CONTROL

EXISTING
PASSIVE
BENCH MARKS

EXISTING PASSIVE
MARKS- HORIZONTAL
WITH OR WITHOUT
VERTICAL

70 KM (NOT TO SCALE)

100 KM (NOT TO SCALE)

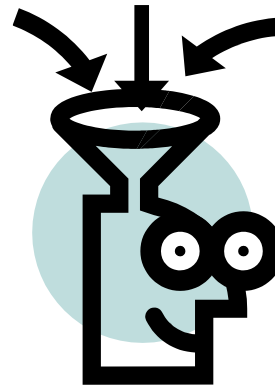
20 KM

NATIONAL CORS =
GEOMETRICAL &
GEOPOTENTIAL
TRUTH



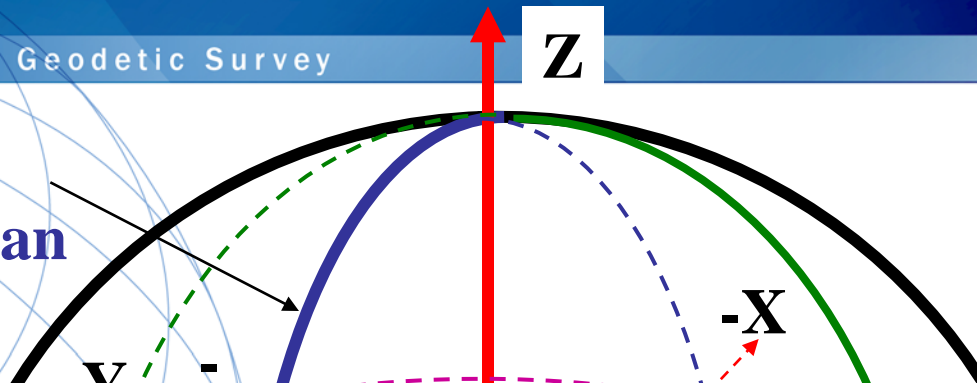
NECESSARY BACKGROUND

(BUT KNOWLEDGE DOES \neq WISDOM)



KNOW YOUR DATUM/ADJUSTMENT/EPOCH

Zero
Meridian



DATUM=
• SURFACE
• ORIENTATION
• SCALE

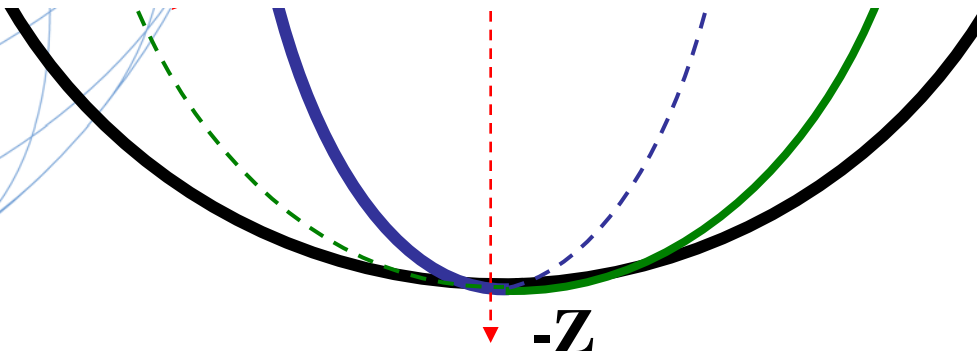
$$a = 6,378,137. \text{ m (exact by definition)}$$

$$1/f = 298.25722210088 \text{ (to 14 significant digits by computation)}$$

From these two numbers, any other desired constants of geometric geodesy may be derived. For example, to 14 significant digits:

$$b = 6,356,752.3141403$$

$$e^2 = 0.0066943800229034.$$



GPS
POSITIONS
ARE ECEF,
XYZ IN THE
WGS 84
DATUM (NOT
NAD 83)

Earth-Centered-Earth-Fixed Coordinates

Zero Meridian

X Axis

Z Axis

Conventional Terrestrial Pole

P (X,Y,Z)

Earth's Surface

Origin
(0,0,0)
Center of Mass

Y Axis

Z

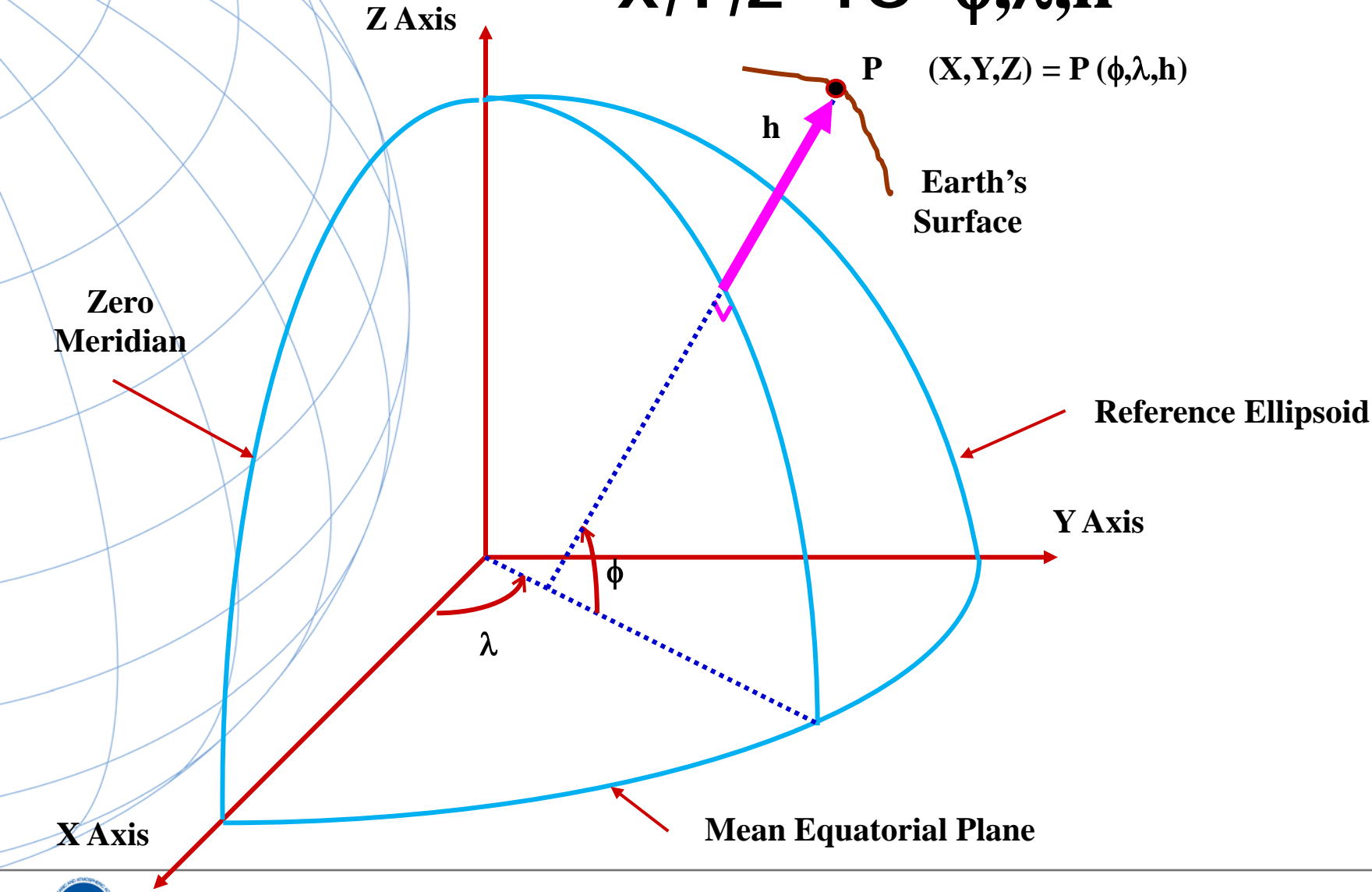
X

Y

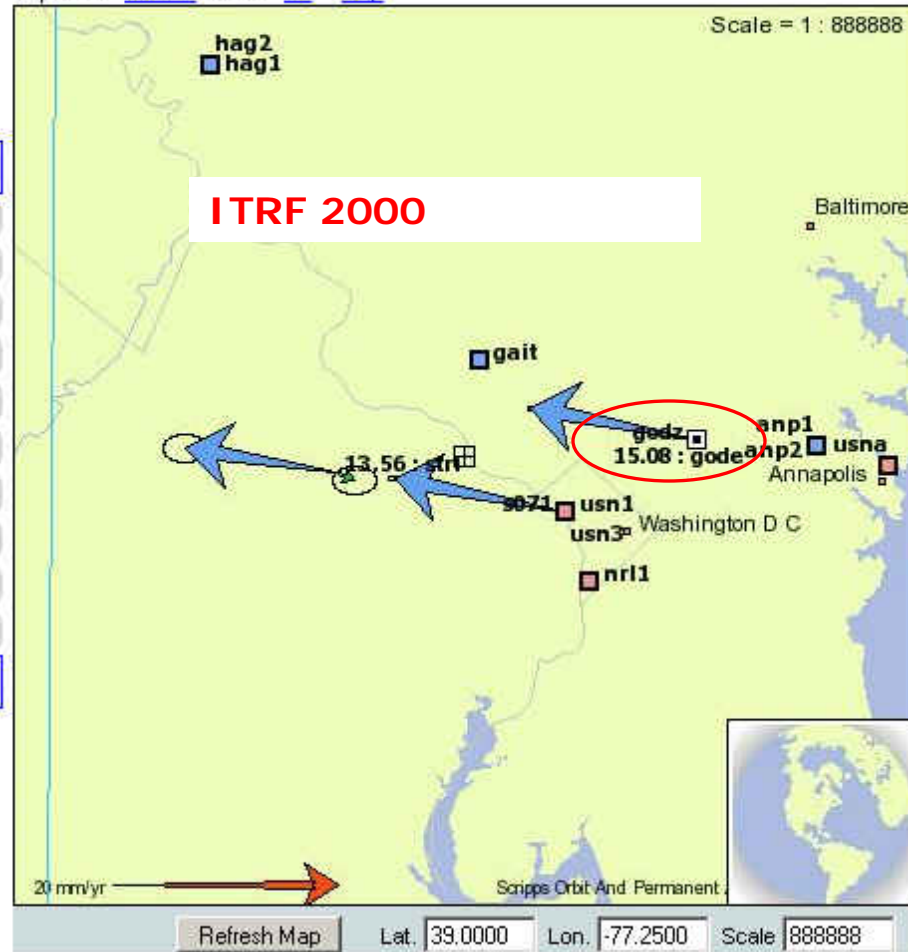
Mean Equatorial Plane



X,Y,Z TO ϕ, λ, h

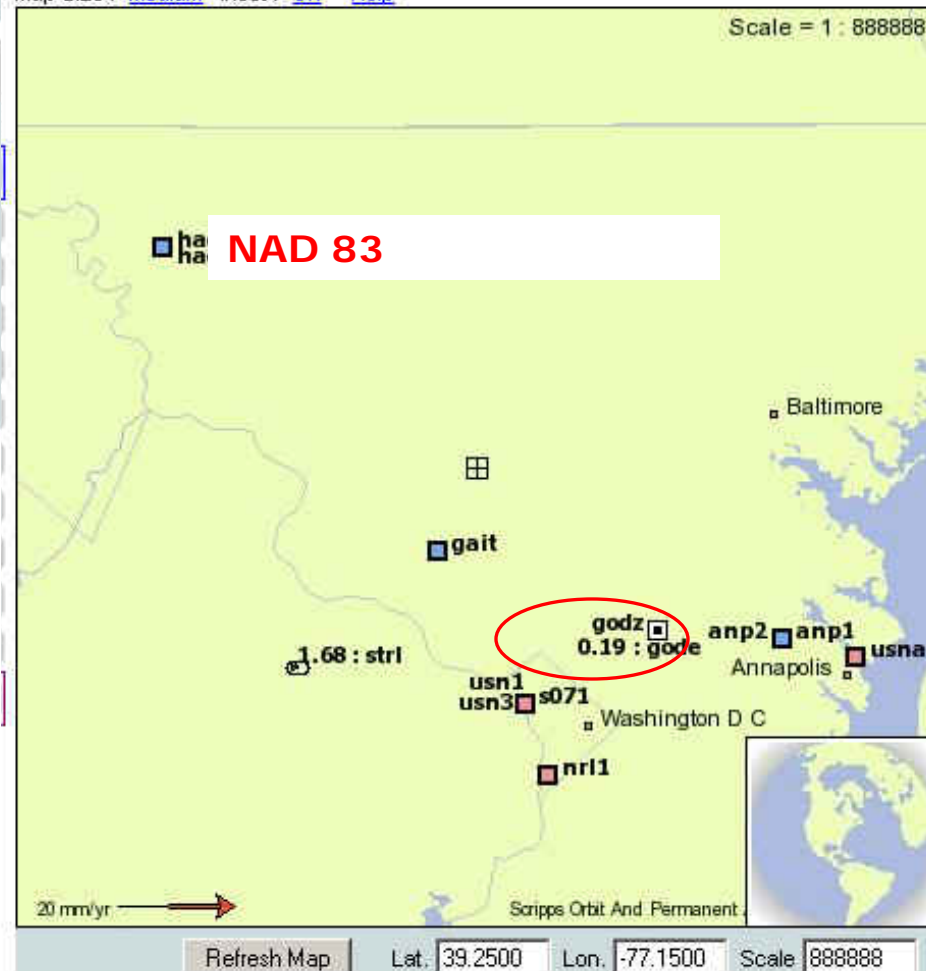


Map Size: [Medium](#) Inset: [On](#) [Help](#)



SILVER SPRING, MD

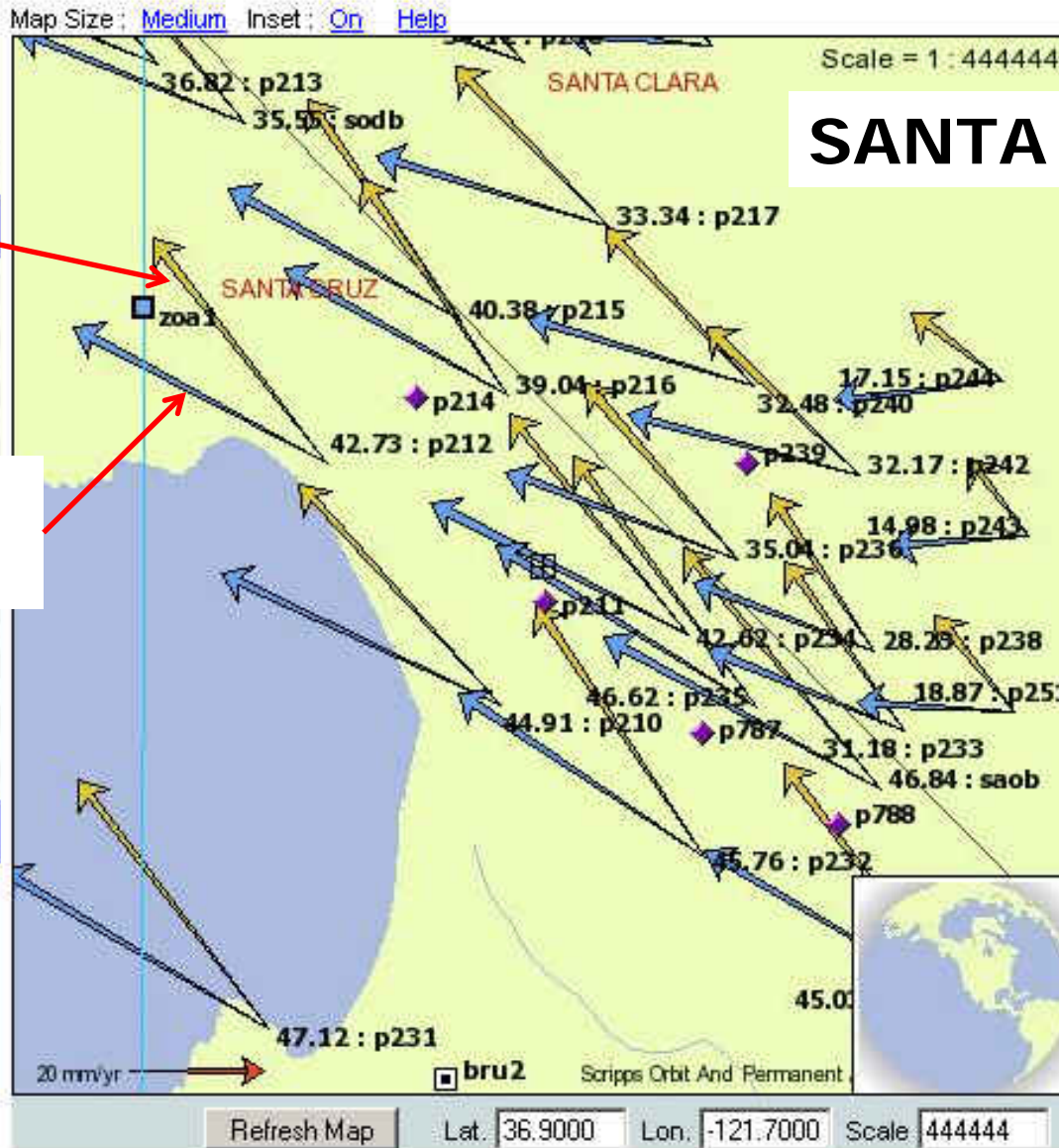
Map Size: [Medium](#) Inset: [On](#) [Help](#)



SANTA CRUZ, CA

NAD 83
VELOCITIES
(GOLD)

ITRF 2000
VELOCITIES
(BLUE)



GNSS TO ANY DATUM

- **GNSS ECEF X,Y,Z (WGS 84 & PZ90)**

→ **NAD 83 (ϕ, λ, h)** → **SPC N,E,h**

+ GEOID XX → **= SPC N,E,H**

OR

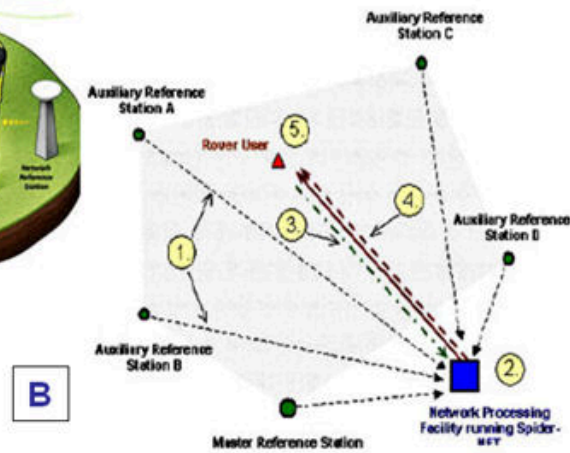
**CALIBRATE TO 4-5 SITE POINTS IN THE
DESIRED DATUM. THIS IS USED TO LOCK TO
PASSIVE MONUMENTATION IN THE PROJECT
AREA.**



USING REAL TIME GNSS NETWORKS FOR POSITIONING

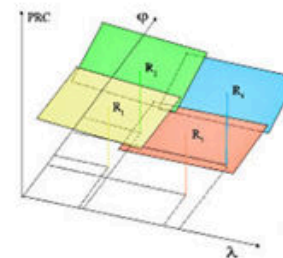


A

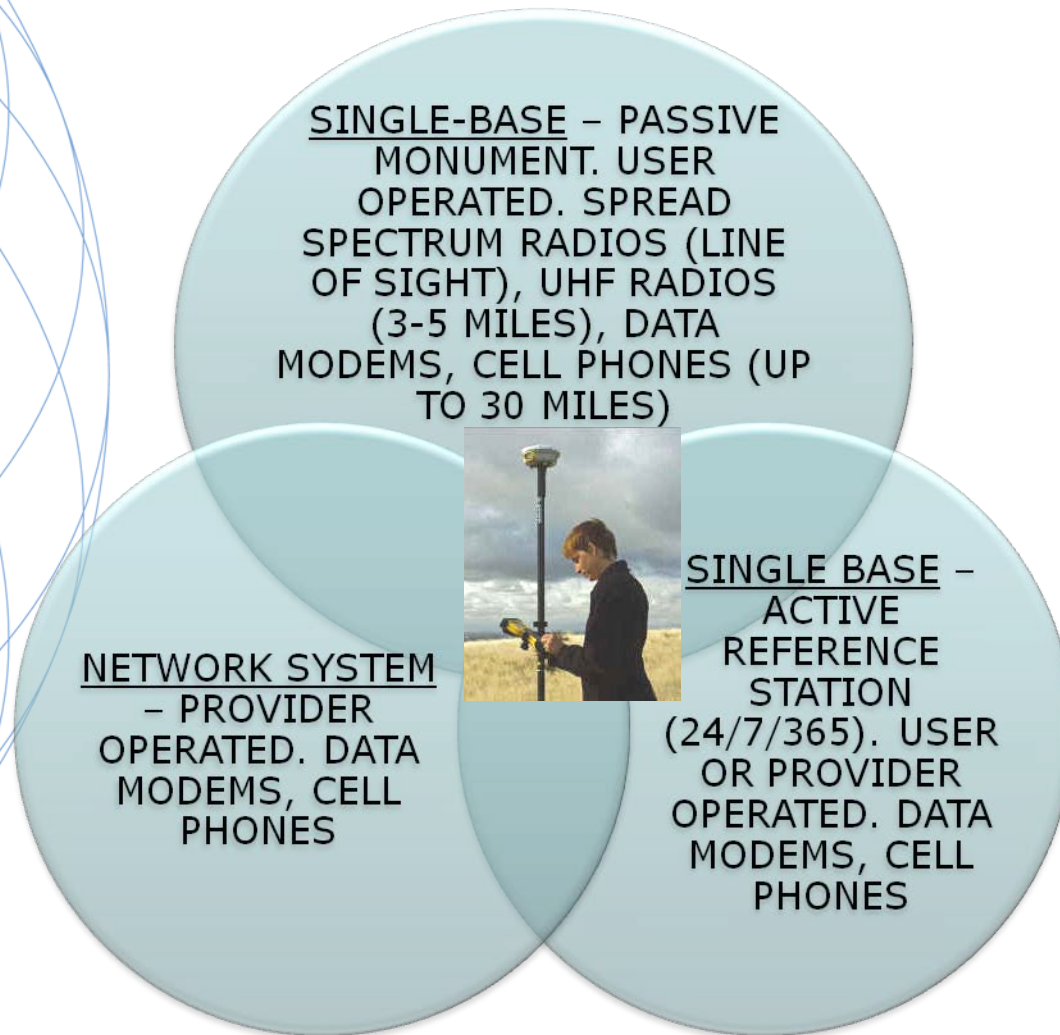


B

C



THE THREE BASE STATION OPTIONS FOR RT



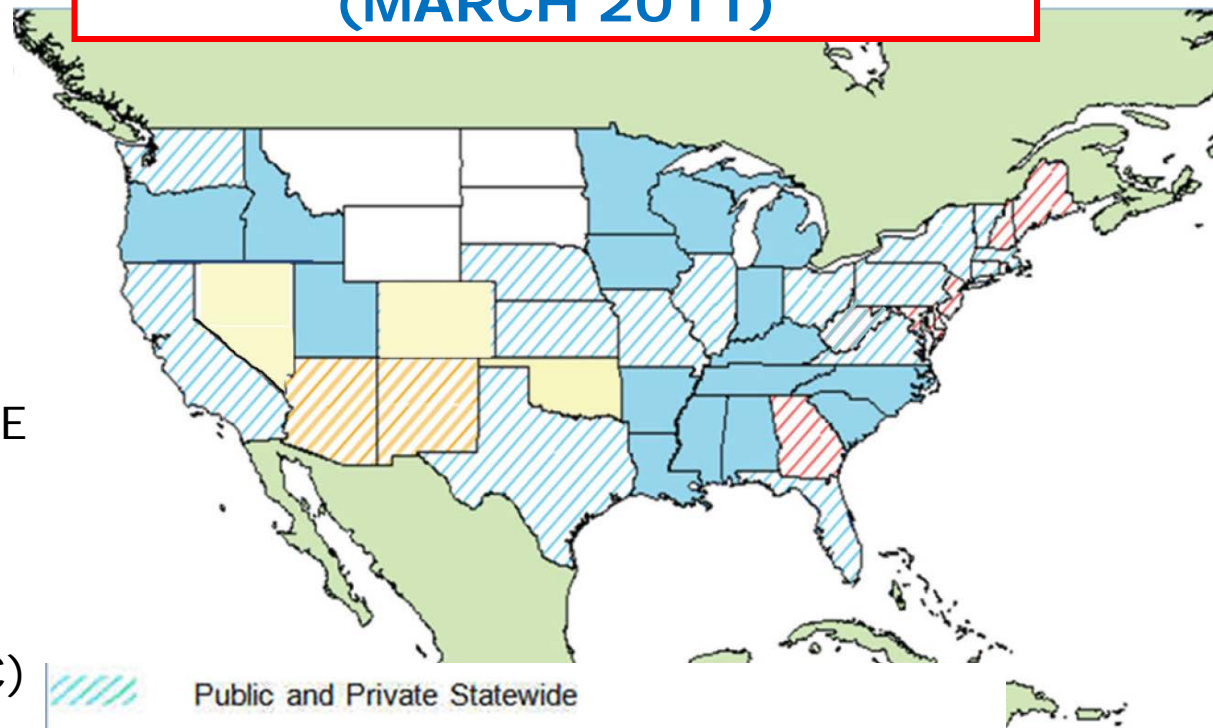
THE TECHNOLOGY SWEET SPOT

- SBAS: 2 M H, 6 M V, 0.3 M SMOOTHED H, CHEAP
- COMMERCIAL DGPS: FEW DM, \$\$
- USCG BEACON: METER+, CHEAP
- DIFFERENTIAL LEVELING: 2-4 CM, LABOR/TIME INTENSIVE, \$\$\$
- GEODETIC LEVELING: mm, LABOR/TIME INTENSIVE, \$\$\$\$\$
- USER BASE RTK: 2-4 CM H, 3-5 CM V, REQUIRES INITIAL INVESTMENT
- RTN: 3-4 CM H, 5-7 CM V, REQUIRES INITIAL INVESTMENT(BUT ½ OF RTK)
- AERIAL MAPPING: .10 M H, .20 M V, \$\$\$
- LIDAR: 0.10 – 0.3 M V
- SATELLITE IMAGERY: 0.5 METER H RESOLUTION, 3 M LOCATION, \$\$\$
- LOW ALTITUDE AERIAL IMAGERY: 2-4 CM H, 3-5 CM V, \$\$\$
- TERRESTRIAL LASER SCANNING: PROJECT SITES ONLY, 0.015 M H, 0.02 M V, REQUIRES INITIAL INVESTMENT



≥200 RTN
WORLDWIDE
≥107 RTN USA
≥37 DOT

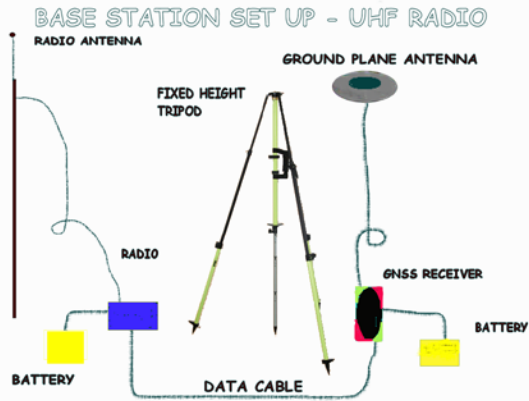
MAJOR RTN IN THE USA (MARCH 2011)



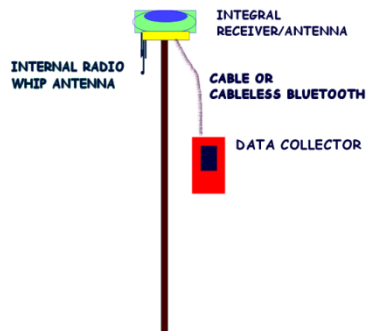
W.Henning 9/2009



RTK vs. RTN



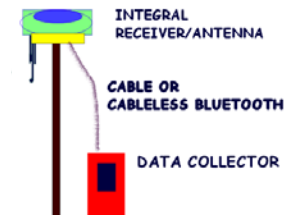
ROVER SET UP - INTERNAL RADIO



RTK

ROVER SET UP

Cell technology



RTN

- Half the equipment or double the production
- No monument reconnaissance/recovery
- No set/break down time
- No base baby sitting

Plus:
Easy alignment to the NSRS
No ppm (1ST ORDER) ERROR
Extended range
Homogeneous Data
Easy datum updates

SO – WHAT CAN I EXPECT FROM An RTN?

MOST RTN PRODUCE “GOOD” HORIZONTAL VALUES – TO A FEW CM. OUR HORIZONTAL SYSTEM IS BASED ON ACTIVE REFERENCE STATIONS (NGS CORS), AS ARE THE RTN STATIONS.

BECAUSE ORTHOMETRIC HEIGHTS (‘ELEVATIONS’) ARE BASED ON PASSIVE MONUMENTS WITH NAVD 88, THE RTN USER SHOULD, FOR THE MOST PART, CONSTRAIN THE PASSIVE MARK VALUES IN A LOCALIZATION.

CHOOSE THE RTN WITH A BUSINESS MODEL THAT BEST FITS YOUR NEEDS.



USERS CONCERNS WITH RTN

- What Datum is the RTN using?
- What adjustment of the Datum is the RTN using?
- What epoch of the Datum adjustment is the RTN using?
- How Does The RTN Align To The NSRS?
- Can Users Use Any Manufacturers' Equipment In The RTN?
- Do Overlapping Networks Give The Same Coordinates?
- What Are The Field Accuracies?



**OVERLAPPING RTN-
NSRS?,
HOMOGENEOUS?, USES
ALL GNSS GEAR?**

TRIMBLE

TRIMBLE

LEICA

LEICA

LEICA

**TRIMBLE
TOPCON**

LEICA

TRIMBLE

**LEICA
TOPCON
TRIMBLE**

TRIMBLE

TOPCON

TRIMBLE



NATIONAL SPATIAL REFERENCE SYSTEM (NSRS)

Consistent National Coordinate System

- Latitude
- Longitude
 - Height
 - Scale
 - Gravity
- Orientation

and how these values change with time

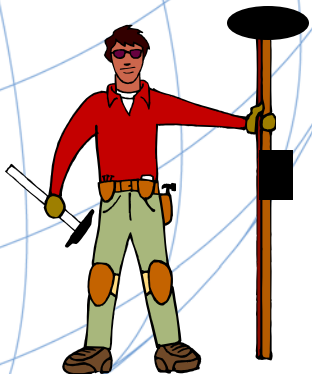


HOW WILL NGS HELP THE USER OF RTN?

“Develop guidelines for both the administration and use of real-time GNSS networks and especially for ensuring that these networks are compatible with the NSRS.”



1. TOP DOWN: OPUS POSITIONS ON RTN REFERENCE STATIONS AT APPROPRIATE INTERVALS COULD PRODUCE GRAPHICS THAT WOULD SHOW BIASES AT A GLANCE.



2. USER UP: PHYSICAL MONUMENTATION, ESTABLISHED WITH BEST TECHNOLOGY, COULD BE USED AS FIDUCIAL STATIONS TO HELP THE USER VERIFY THAT RTN ARE PRODUCING ACCURATE COORDINATES,

(NEAR) REAL TIME GNSS POSITIONING – BEYOND THE BLACK BOX



HOW DOES RT WORK?

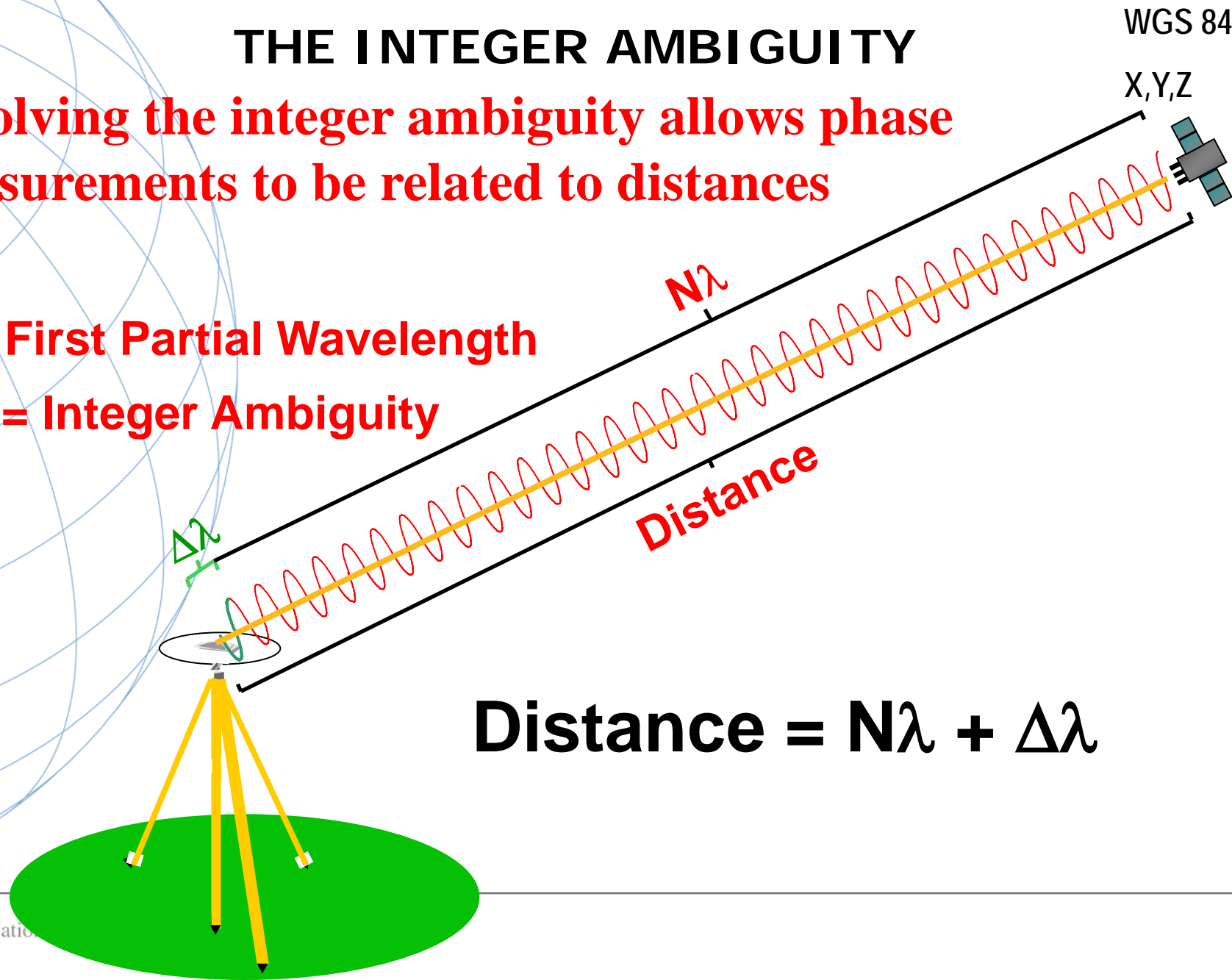
- $\Delta X, Y, Z$ FROM BASE (REMEMBER "GIGO")
- $\text{MULTILATERATION} - \text{TIME (SEC.)} \cdot C \text{ (SPEED OF LIGHT) (M/SEC.)} = \text{DISTANCE from satellite}$
- MUST RESOLVE CARRIER CYCLE INTEGER COUNT AMBIGUITIES ($\# \text{ cycles} \cdot \text{wave length} + \text{partial cycle} = \text{distance}$)
- MUST ACCOUNT FOR FACTORS AFFECTING THE PATH OF THE SIGNAL
- DUAL FREQUENCY ENABLES "ON THE FLY" RESOLUTION OF THE AMBIGUITIES & EASIER CYCLE SLIP DETECTION THAN L1 ONLY
- FREQUENCY COMBINATIONS AND DIFFERENCING CONTRIBUTE TO MITIGATING THE ERROR BUDGET

THE INTEGER AMBIGUITY

Resolving the integer ambiguity allows phase measurements to be related to distances

$\Delta\lambda$ = First Partial Wavelength

$N\lambda$ = Integer Ambiguity



THE AMBIGUITY SEARCH....

The ambiguity is an *integer* number (a multiple of the carrier wavelength).

The integer is different for the L1 and L2 phase observations.

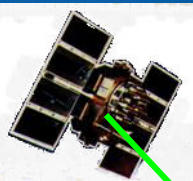
The integer ambiguity is different for each *satellite-receiver pair*.

The integer ambiguity is a constant for a particular satellite-receiver pair for all epochs of *continuous* tracking (that is, as long as no **cycle slips** occur)

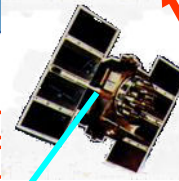
The carrier phase measurement from one observation epoch to another is a measure of the *change* in satellite-receiver range.

The determination of the cycle ambiguity integer is known as **ambiguity resolution**, and is generally not an easy task because of the presence of other biases and errors in the carrier phase measurement.

8400 MPH

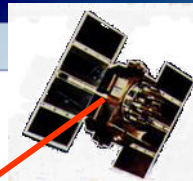


SVN 31



SVN 11

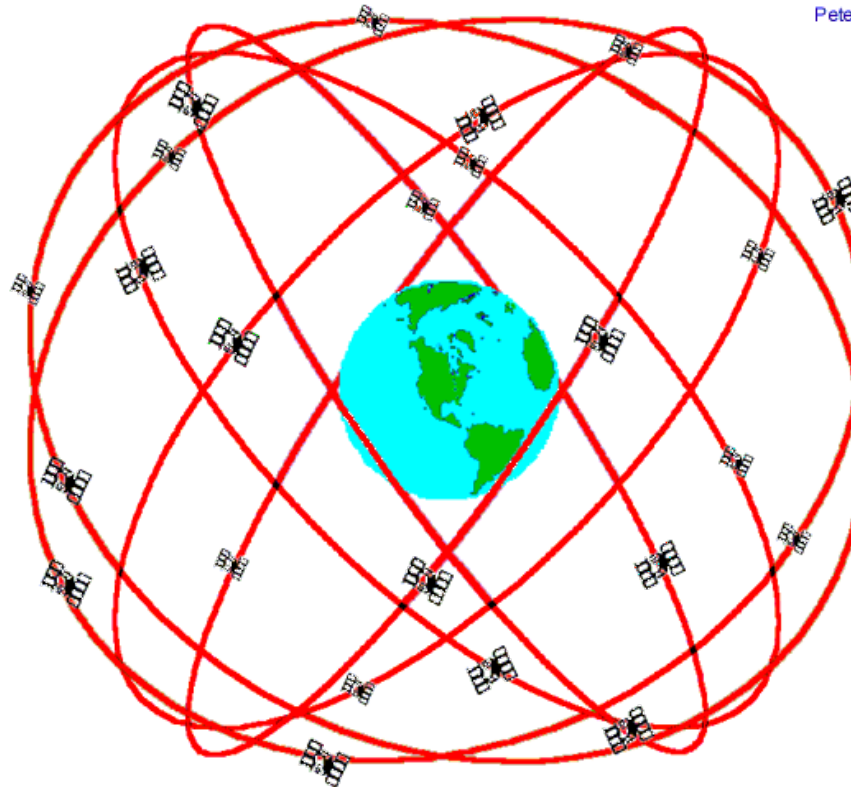
8400 MPH



SVN 8

8400 MPH

EVERY EPOCH OF
OBSERVATION
MUST COMPUTE
ANGLE IN
POSITION DUE TO
EARTH ROTATION,
SATELLITE ORBIT
ANGLE AND
LATITUDE!



GPS Nominal Constellation
24 Satellites in 6 Orbital Planes
4 Satellites in each Plane

20,200 km Altitudes, 55 Degree Inclination
AT "C", 1 NANOSECOND = 30 CM!

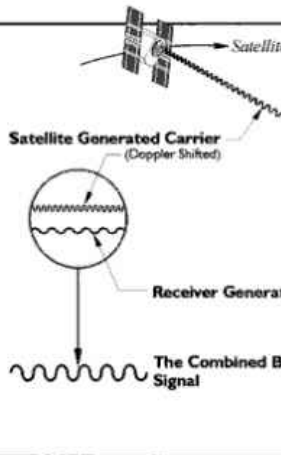
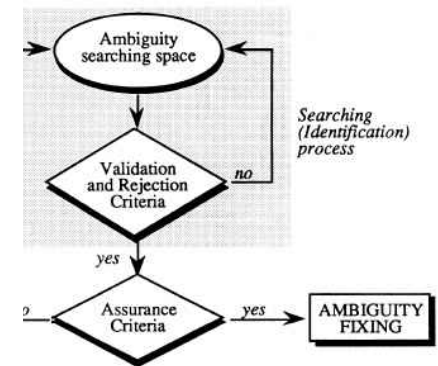


Figure 1.10. The Carrier Beat Phase.



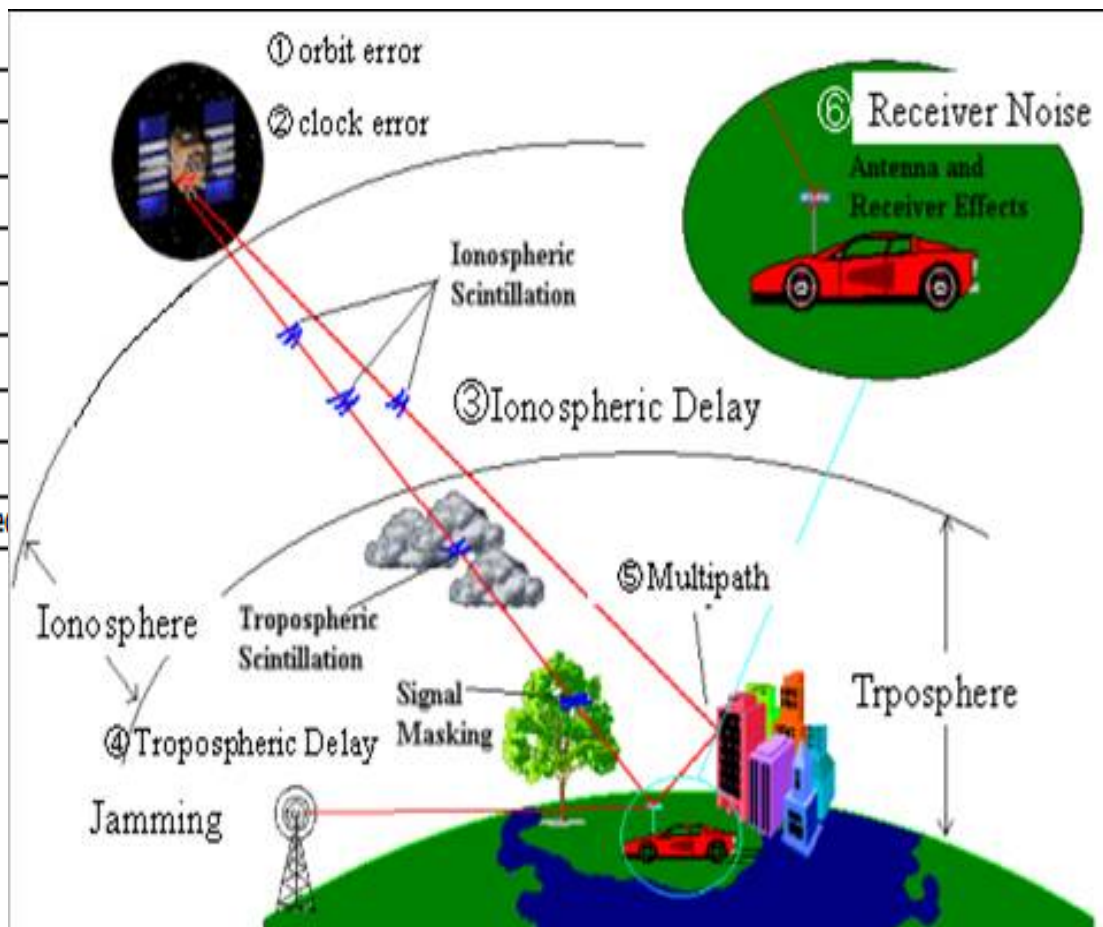
General strategy of on-the-fly ambiguity resolution.

RT PROCESSING IN YOUR ROVER

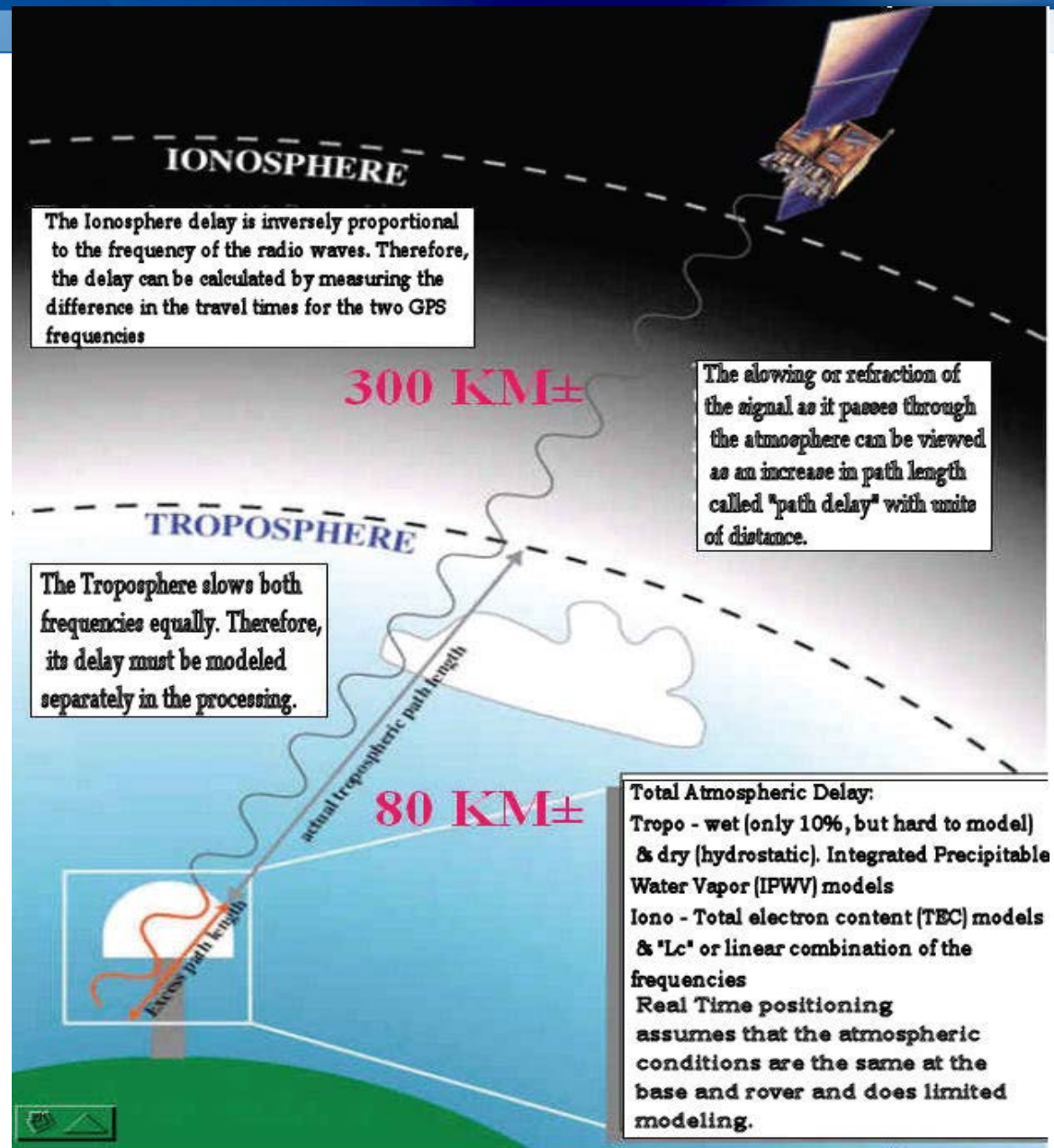
UNDIFFERENCED PHASE OBSERVABLE (CYCLES)

$$\varphi_k^p(t) = \frac{f}{c} \rho_k^p(t) - f \Delta t_k(t) + f \Delta t^p(t) + N_k^p - I_{k,\varphi}^p(t) + \frac{f}{c} T_k^p(t) + d_{k,\varphi}(t) + d_{k,\varphi}^p(t) + d_{\varphi}^p(t) + \varepsilon_{\varphi}$$

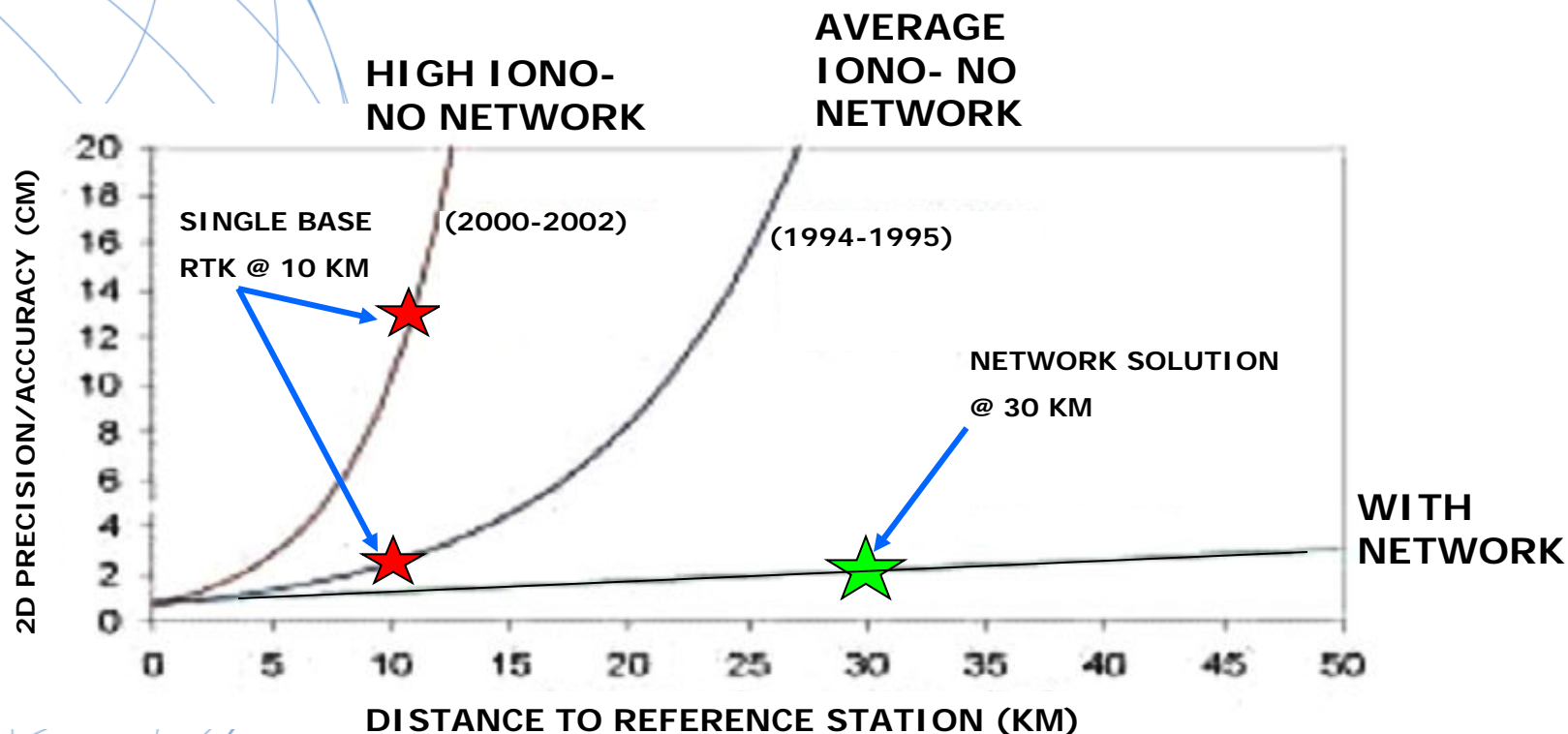
ERROR	VALUE
Ionosphere	4.0 METERS
Ephemeris	2.1 METERS
Clock	2.1 METERS
Troposphere	0.7 METERS
Receiver	0.5 METERS
Multipath	1.0 METERS
TOTAL	10.4 METERS
UNCORRELATED ERROR	5.15 m (square root of sum of errors square)



IONO & TROPO LAYERS AND THEIR EFFECT ON THE GNSS SIGNAL



IONOSPHERIC EFFECTS ON POSITIONING

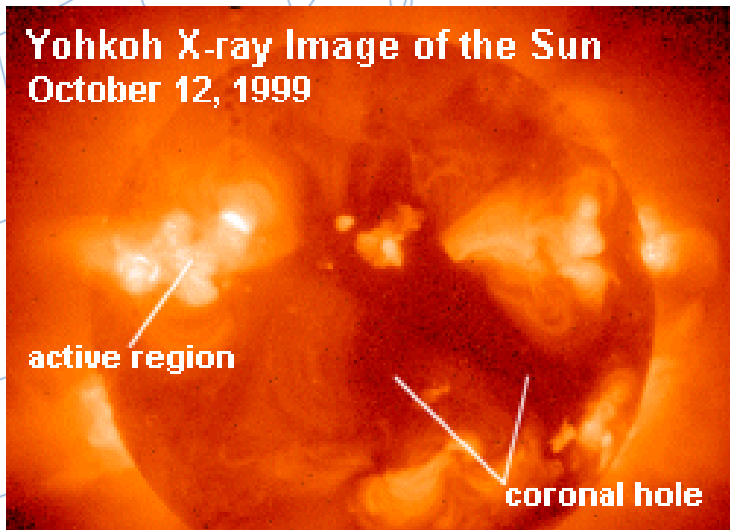


(SOURCE-BKG- GERMANY)

SUNSPOT CYCLE

- Sunspots follow a regular 11 year cycle
- We are just past the low point of the current cycle
- Sunspots increase the radiation hitting the earth's upper atmosphere and produce an active and unstable ionosphere

Yohkoh X-ray Image of the Sun
October 12, 1999



ISFC Solar Cycle, Sunspot Number Progression
SOLAR CYCLE – SUNSPOTS AND FLARES

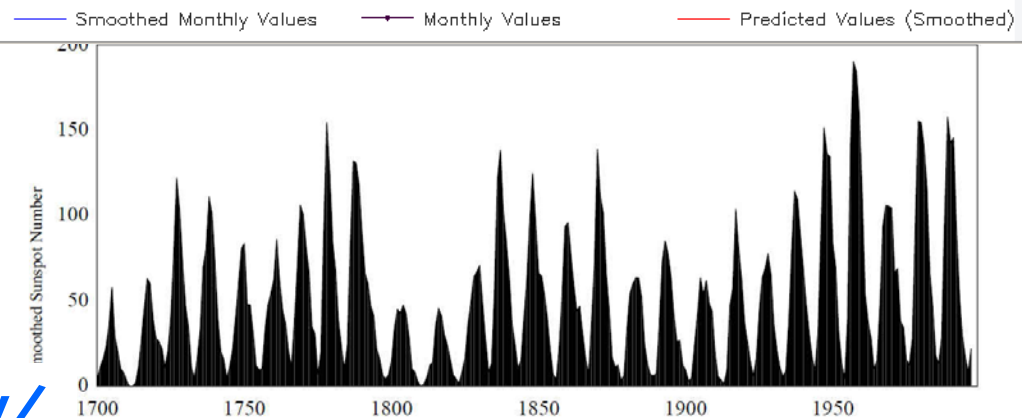
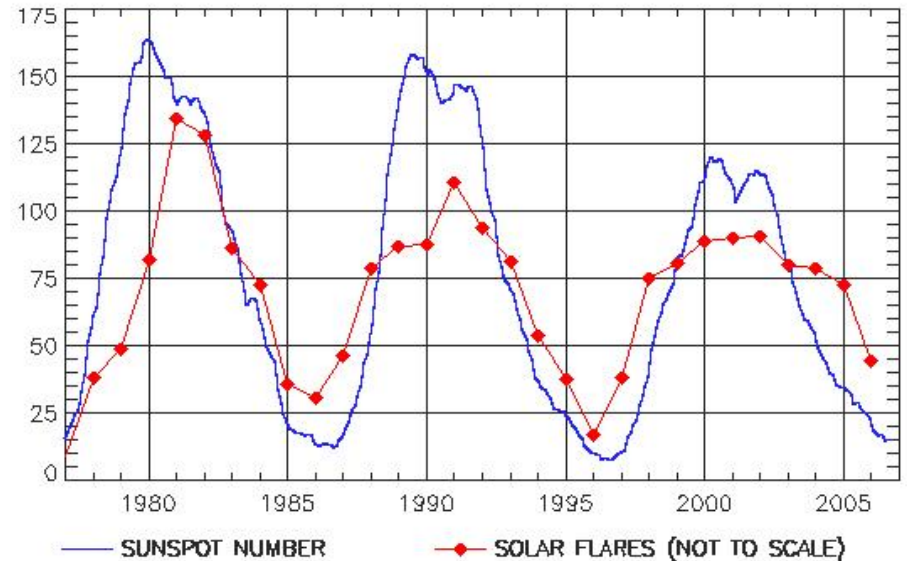


Figure 1. The Sunspot Cycle, well documented over the last 300 years, reveals a 10-11 year pattern of solar activity.

Official Space Weather Advisory issued by NOAA Space Weather Prediction Center
Boulder, Colorado, USA SPACE WEATHER ADVISORY BULLETIN #10- 1 2010 April
05 at 12:13 p.m. MST (2010 April 05 1213 UTC) **** STRONG GEOMAGNETIC

{ :Product: Geophysical Alert Message www.txt

/ :I **SINGLE FREQUENCY USERS USE A MODEL FOR IONO**
(# **CORRECTIONS, SO DURING GEOMAGNETIC STORMS,**
(# **THEY WILL EXPERIENCE MORE DRAMATIC ERROR AND**
/ # **NOISE THAN DUAL+ FREQUENCY USERS WHO MAY**
S So **USE THE DISPERSIVE CHARACTER OF THE**
S So **IONOSPHERE TO CALCULATE THE ACTUAL ERROR.**
i Th

(Space weather for the past 24 hours has been strong.
(Geomagnetic storms reaching the G3 level occurred.

{ Space weather for the next 24 hours is expected to be minor.
{ Geomagnetic storms reaching the G1 level are expected.



TROPOSPHERE DELAY

The more air molecules, the slower the signal (dry delay)

High pressure, Low temperature

90% of total delay

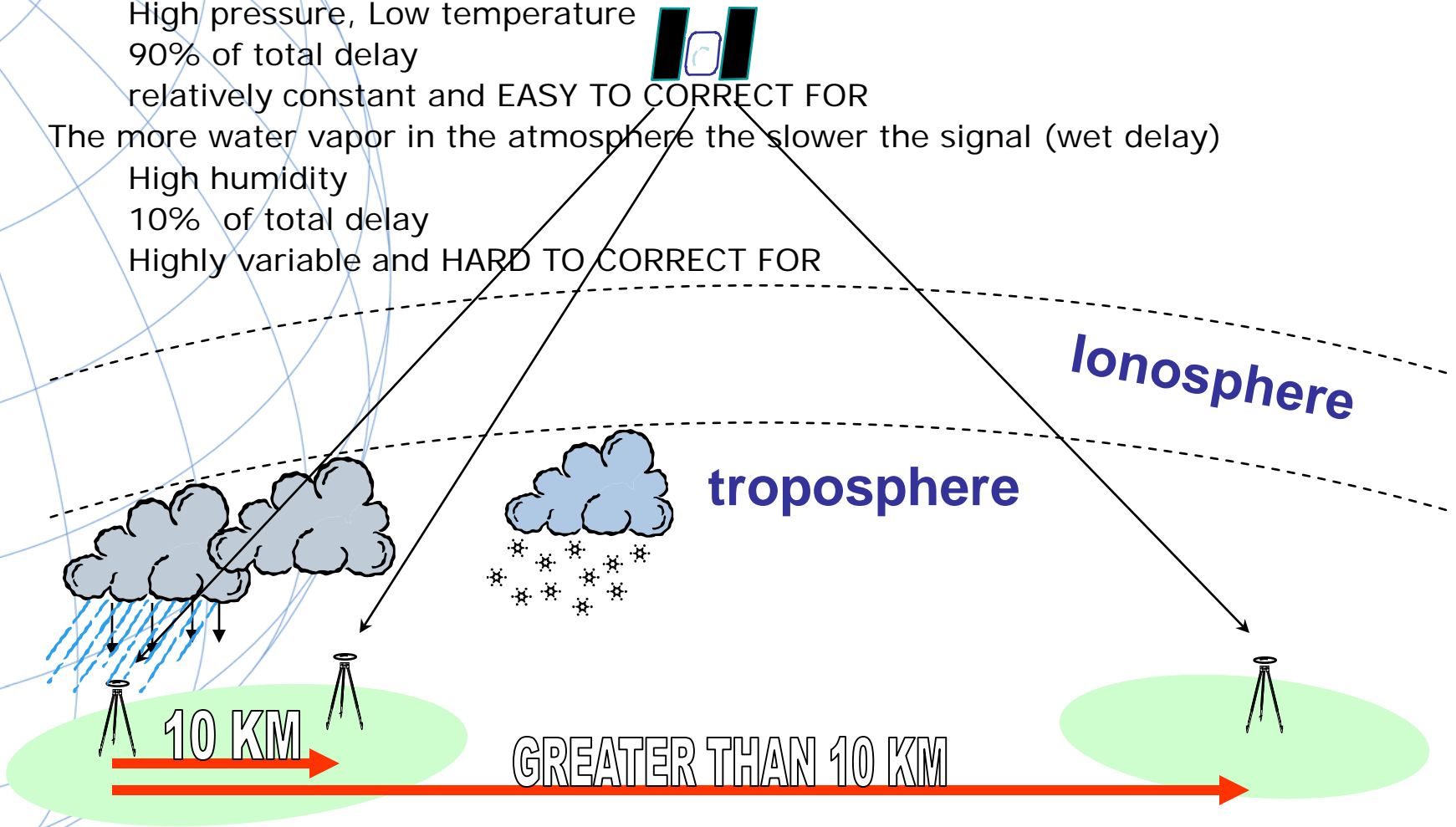
relatively constant and EASY TO CORRECT FOR

The more water vapor in the atmosphere the slower the signal (wet delay)

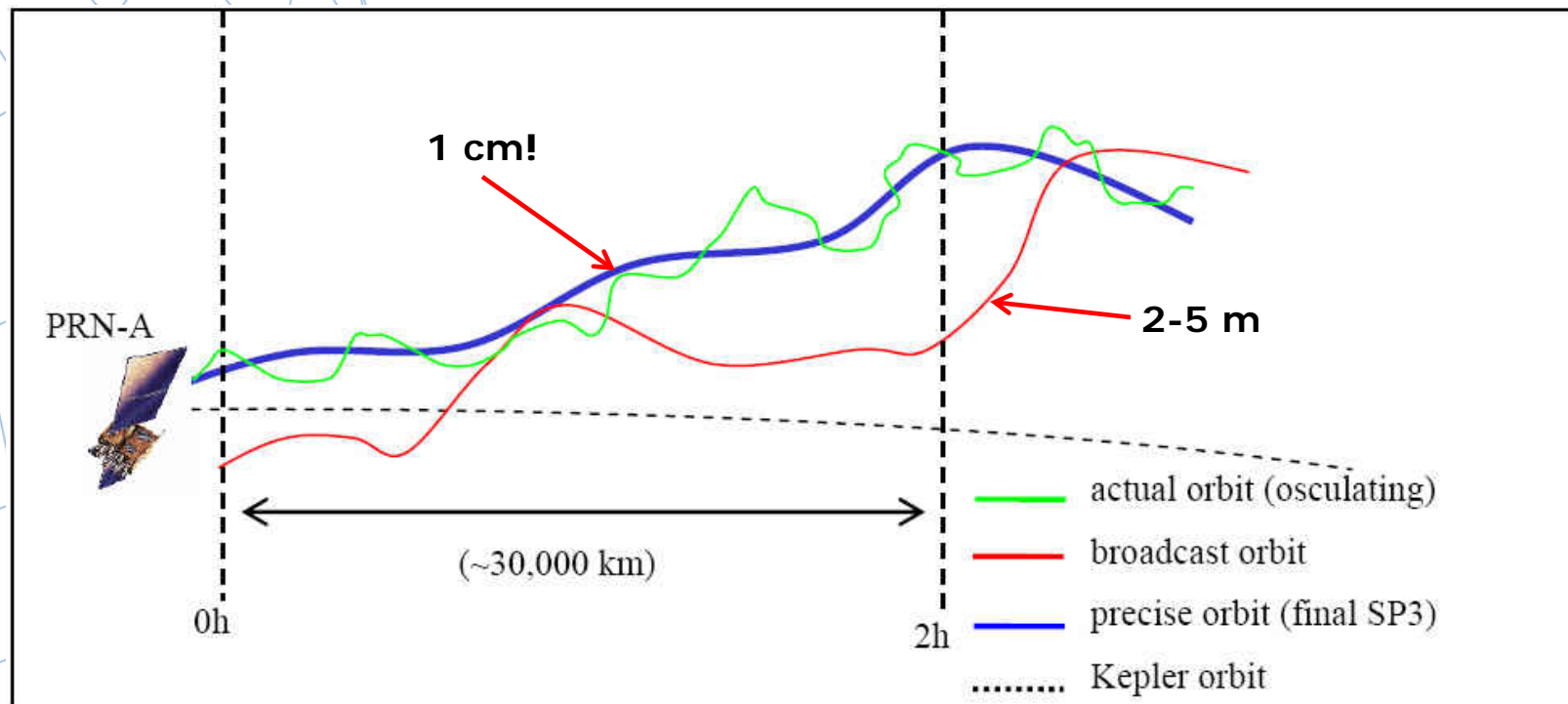
High humidity

10% of total delay

Highly variable and HARD TO CORRECT FOR



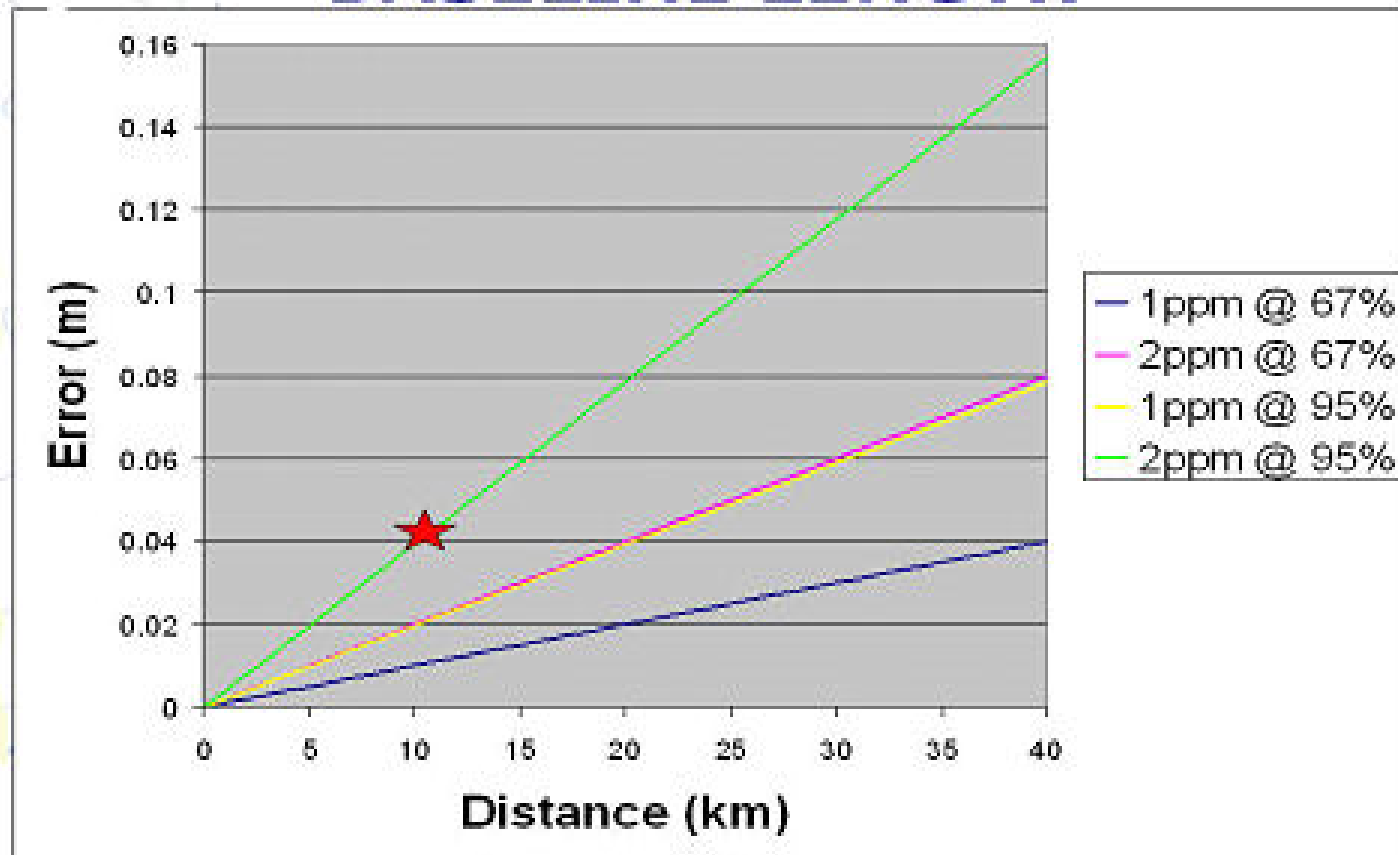
ORBITAL ERRORS CONTRIBUTING TO PPM ERRORS



(See user guidelines
references for
Graphic: Ahn, 2005)

IONO, TROPO, ORBIT CONTRIBUTE TO PPM ERROR

RTK PPM ERROR VS. BASELINE LENGTH



REMEMBER GNSS EQUIPMENT MANUFACTURERS' SPECS!

THE UNDIFFERENCE CARRIER PHASE OBSERVABLE IN CYCLES

$$\varphi_k^p(t) = \frac{f}{c} \rho_k^p(t) - f dt_k(t) + f dt^p(t) + N_k^p - I_{k,\varphi}^p(t) + \frac{f}{c} T_k^p(t) + d_{k,\varphi}(t) + d_{k,\varphi}^p(t) + d_\varphi^p(t) + \varepsilon_\varphi$$

Superscripts refer to the satellite, subscripts refer to ground station

φ : Carrier phase observable in cycles φ_k^p refers to the carrier phase observable from SV p to Station k.

f : Carrier frequency

c : Speed of light

$\rho(t)$: The topocentric range ρ_k^p is the range from SV p to Station k.

$dt_k(t)$: Receiver clock error as a function of time

$dt^p(t)$: SV clock error as a function of time

N_k^p : The integer ambiguity from SV p to Station k

$I_k^p(t)$: Ionospheric advance $I_{k,\varphi}^p$ is the Ionospheric advance from SV p to Station k in cycles

$T(t)$: Tropospheric delay T_k^p is the tropospheric delay from SV p to Station k

$d_{k,\varphi}(t)$: Receiver hardware delays in cycles as a function of time

$d_{k,\varphi}^p(t)$: Multipath in cycles as a function of time

$d_\varphi^p(t)$: Satellite hardware delays in cycles as a function of time

ε_φ : Measurement noise in cycles



THE CYCLE COUNT COOKBOOK-

USING **DIFFERENCING** TO ELIMINATE OR REDUCE COMMON ERRORS IN THE RECEIVER AND SATELLITE

$$\phi_k^p(t) = \frac{f}{c} \rho_k^p(t) - f dt_k(t) + f dt^p(t) + N_k^p - I_{k,\phi}^p(t) + \frac{f}{c} T_k^p(t) + d_{k,\phi}(t) + d_{k,\phi}^p(t) + d_\phi^p(t) + s_\phi$$

- RECEIVER HARDWARE DELAYS
- SATELLITE HARDWARE DELAYS
- RECEIVER CLOCK BIAS
- SATELLITE CLOCK BIAS

ELIMINATED WITH DIFFERENCING

- IONO DELAY
- TROPO DELAY

SAME AS BASE WITH SINGLE BASE INTERPOLATED WITH RTN

- MEASUREMENT NOISE (HIGHER GRADE RECEIVERS = LESS NOISE)
- MULTIPATH

NOT ELIMINATED WITH DIFFERENCING

RESULTING DIFFERENCED PHASE OBSERVABLE (CYCLES)

$$\phi_k^p(t) = \frac{f}{c} \rho_k^p(t) - \cancel{\int \dot{a}_{k,p}(t) dt} - \cancel{f \dot{a}_{k,p}(t)} - \cancel{I_{k,p}^p(t)} - \boxed{I_{k,p}^p(t) + \frac{f}{c} T_k^p(t)} + \cancel{a_{k,p}(t)} + \cancel{\dot{a}_{k,p}(t)} + \cancel{\ddot{a}_{k,p}(t)} + \varepsilon_p$$

Superscripts refer to the satellite, subscripts refer to ground station

**LEAVES MULTIPATH,
MEASUREMENT NOISE
& RANGE TO
SATELLITE**

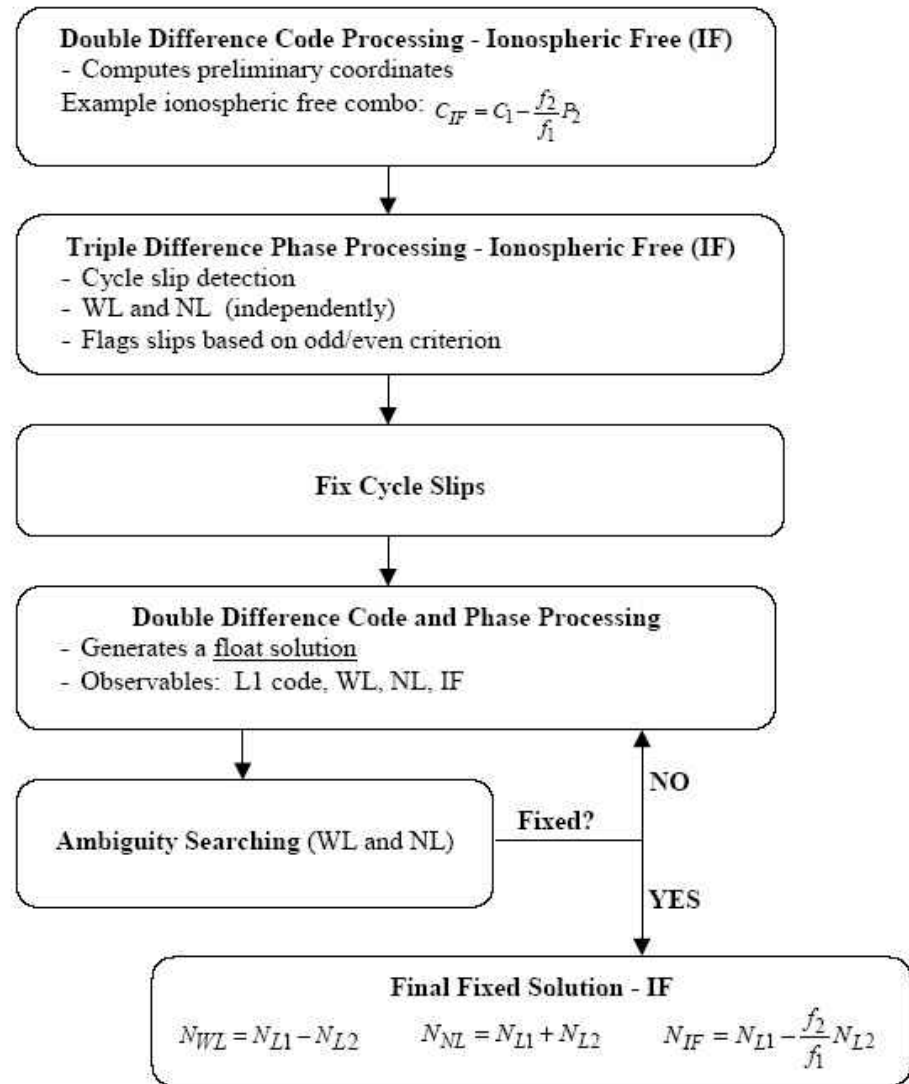
**ASSUMED THE SAME FOR
ROVER & BASE**

OR MODELED BY RTN

CARRIER PHASE PROCESSING FLOW

REFERENCE:

“Parameterization of DGPS Carrier Phase Errors Over a Regional Network of Reference Stations”
(URL: <http://www.geomatics.ucalgary.ca/GradTheses.html>)
by
Georgia Fotopoulos
August 2000





BEST METHODS FOR REAL TIME POSITIONING



REAL-TIME CHOICES - BIG PICTURE ISSUES

- **PASSIVE / ACTIVE – WHAT IS ‘TRUTH’?**
- **GEOID + ELLIPSOID / LOCALIZE –**
QUALITY OF GEOID MODELS LOCALLY.
ORTHOMETRIC HEIGHTS ON CORS?
- **GRID / GROUND –**
LOW DISTORTION PROJECTIONS- SHOULD NGS PLAY?
- **ACCURACY / PRECISION- IMPORTANCE OF METADATA**
- **SINGLE SHOT / REDUNDANCY**
- **RTK / RTN**
- **NATIONAL DATUMS / LOCAL DATUMS / ADJUSTMENTS-**
DIFFERENT WAYS RTN GET THEIR COORDINATES-
VARIOUS OPUS, OPUS-DB, CORS ADJUSTED, PASSIVE
MARKS.
VELOCITIES - NEW DATUMS, “4 -D” POSITIONS
- **GNSS / GPS**

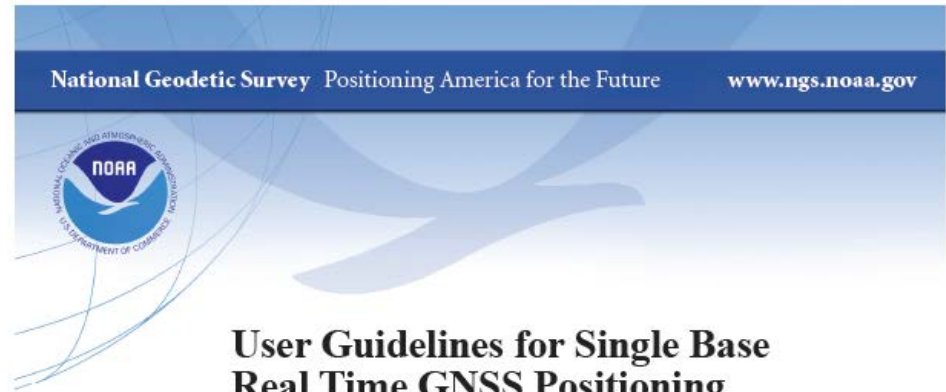


FOUR CARDINAL RULES FOR RT POSITIONING

- COMMUNICATIONS: THE KEY TO SUCCESS
- CHECK SHOT: FIRST BEFORE NEW WORK
- REDUNDANCY: FOR CONFIDENCE
- MULTIPATH: AVOID UNSUITABLE CONDITIONS

NGS SINGLE BASE GUIDELINES

- LEGACY EQUIPMENT
- NO CELL COVERAGE
- NEW RT CLOSEST BASE NETWORKS
- MACHINE GUIDANCE AND PRECISION AGRICULTURE USE



William Henning, Lead Author

http://www.ngs.noaa.gov/PUBS_LIB/NGSRealTimeUserGuidelines.v2.1.pdf

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RT USER GUIDELINES

DRAFT GUIDELINES- 95% CONFIDENCE

ACCURACY CLASS SUMMARY TABLE

	CLASS RT1	CLASS RT2	CLASS RT3	CLASS RT4
ACCURACY (TO BASE)	0.015 HORIZONTAL, 0.025 VERTICAL	0.025 HORIZONTAL, 0.04 VERTICAL	0.05 HORIZONTAL, 0.06 VERTICAL	0.15 HORIZONTAL, 0.25 VERTICAL
REDUNDANCY	≥ 2 LOCATIONS, 4-HOUR DIFFERENTIAL	≥ 2 LOCATIONS, 4-HOUR DIFFERENTIAL	NONE	NONE
BASE STATIONS	≥ 2, IN CALIBRATION PROJECT CONTROL	RECOMMEND 2 IN CALIBRATION	≥ 1, IN CALIBRATION	≥ 1, IN CALIBRATION RECOMMENDED
PDOP	≤ 2.0	≤ 3.0	≤ 4.0	≤ 6.0
RMS	≤ 0.01 M	≤ 0.015 M	≤ 0.03 M	≤ 0.05 M
COLLECTION INTERVAL	1 SECOND FOR 3-MINUTES	5 SECONDS FOR 1-MINUTE	1 SECOND FOR 15 SECONDS	1 SECOND FOR 10 SECONDS
SATELLITES	≥ 7	≥ 6	≥ 5	≥ 5
BASELINE DISTANCE	≤ 10 KM	≤ 15 KM	≤ 20 KM	ANY WITH FIXED SOLUTION
TYPICAL APPLICATIONS	PROJECT CONTROL CONSTRUCTION CONTROL POINTS CHECK ON TRAVERSE, LEVELS SCIENTIFIC STUDIES PAVING STAKE OUT	DENSIFICATION CONTROL TOPOGRAPHIC CONTROL PHOTOPOINTS UTILITY STAKE OUT	TOPOGRAPHY CROSS SECTIONS AGRICULTURE ROAD GRADING SITE GRADING	SITE GRADING WETLANDS GIS POPULATION MAPPING ENVIRONMENTAL



National Geodetic Survey
Guidelines for Real Time GNSS Networks



February 2011
v. 1.6

RTN GUIDELINES FOR GNSS POSITIONING— WILL NOT SPECIFY OR DEFINE A STANDARD, BUT WILL HELP ADMINISTRATORS AND USERS TO BE AWARE OF ALL THE ISSUES INVOLVED WITH THIS NEW TECHNOLOGY

60+ CONTRIBUTORS:

- NGS ADVISORS
 - DOT
- STATE GEODETIC SURVEYS
- GNSS MANUFACTURERS
 - SRCs
 - BLM, NPS

http://www.ngs.noaa.gov/PUBS_LIB/NGS.RTN.Public.v2.0.pdf

VI. RTN User Guidelines

A. Benefits to the user of an RTN over single-base Real Time (RT) include:

1. No user base station is necessary. Therefore, there are with the base, no control recovery is necessary to establish and the user needs only half the equipment to produce RT (conversely, one can double productivity). Additionally, the time setting up and breaking down the base station equipment is reduced.
2. The first order ppm (part per million) error is eliminated (reduced) because ionospheric, tropospheric and orbital errors are interpolated to the site of the rover. This enables centimeter positioning at extended ranges over 10 km from a reference station.
3. The network can be aligned with the NSRS with high accuracy. Users will then be collecting positional data that will fit together seamlessly across the RTN coverage. This is important to geospatial data, such as GIS professionals who by using the network may deal with such regional issues as emergency management security issues.
4. Datum readjustments or changes can be done transparently with no post campaign work. New datum adjustments to transformations to another geodetic datum such as the International Terrestrial Reference Frame (ITRF) or the forthcoming next generation NAD 83 (NGS, 2008) are done at the network level and not by the users.
5. With some business models, the user can share in the cost of installing a network reference station and getting a share of the subscription fees imposed upon other network users.
6. Different formats and accuracies are readily available. Environmental resource data, mapping grade data, etc. can be collected with 30 to 60 centimeter accuracy while surveyors and engineers can access the network with one centimeter level accuracy. Real-time and other binary formats can be user selected.
7. The RTN can be quality checked and monitored in real-time using utilities such as OPUS from NGS (<http://www.ngs.noaa.gov/OPUS/>) and TEQC from UNAVCO (<http://facility.unavco.org/software/teqc/teqc.html>).

B. Drawbacks to the user of an RTN compared to classical RT include:

1. Network subscription fees. These may be prohibitive for small companies – even though there are considerable savings in equipment.

F. Best Methods for RTN Users – The “Seven C’s” of NOAA’s NGS

This section in conjunction with the information and recommendations contained in the existing NGS guidelines for RT single base positioning (Henning, 2008), is given as a means to produce accurate, repeatable, homogenous positions at the 95% confidence level. It is understood that other methodology may produce similar results; however, these guidelines are given as a confident path that will lead to the stated precision and accuracy.

1. Check Equipment, Data Collector Parameters & Site information

- a) Measure the actual height of the antenna reference point (ARP) on the rover pole at the start of a campaign.
- b) Ensure that all necessary and correct projection parameters are in the data collector.
- c) Ensure that all project data are in the data collector. It is critical that the correct project calibration (a.k.a. localization), if any, is being used. Project control coordinates must be current.
- d) Adjust the rover pole bubble before every campaign.
(see http://www.surveying.com/tech_tips/details.asp?techTipNo=19 “Adjustment of the Circular Vial”) (SECO, 2009a)
(see http://www.surveying.com/tech_tips/details.asp?techTipNo=13 for pole bubble calibration). (SECO, 2009b)
- e) Test wireless data communications (cell/CDMA/SIM card/etc.) for Internet connectivity at the project site.
- f) Make sure the GNSS unit and the communication device batteries are fully charged and that there are backups.
- g) For orthometric heights, be sure to preload the current geoid model supplied by the NGS. To obtain a reasonably sized file for upload, a regional section of the national model can be extracted in all major GNSS manufacturers’ software packages.

2. Conditions

- a) Use mission planning in the GNSS manufacturer’s software to assess approximate times of poor DOP and/or low number of satellites. The Russian GLONASS constellation is on track to have full operational capability by the end of 2010. While there is little improved accuracy at the present time using the GLONASS constellation, it does enable RT positioning where GPS alone would not permit because of an inadequate number of satellites.

“CONFIDENCE” IN YOUR POSITION INCREASES WITH:

- MORE SATELLITES
- SHORTER BASELINES
- LOWER ‘DOP’
- MORE OPEN SKY
- LOWER RMS
- CONTINUOUS COMMUNICATION
- REDUNDANCY, REDUNDANCY, REDUNDANCY

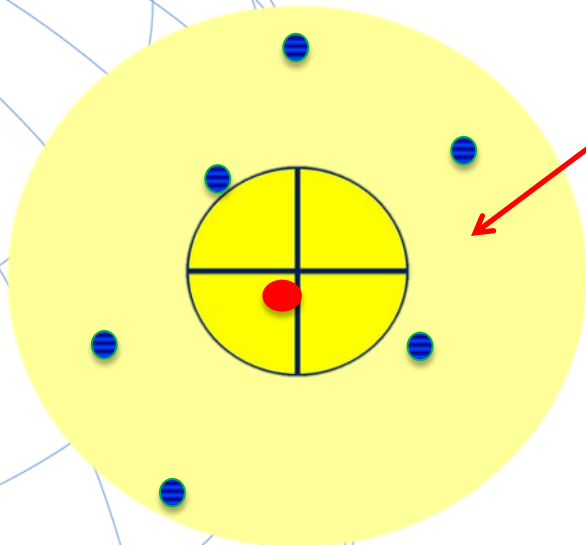
(THE BEST OF ALL SINGLE BASE WORLDS = 8 GPS SATELLITES, GDOP 1.5, 2 KM BASELINE, $RMS \leq 0.01$ M, OPEN SKY, NO WEATHER ELEMENTS, SOLID COMMUNICATION)

PRECISION VS. ACCURACY

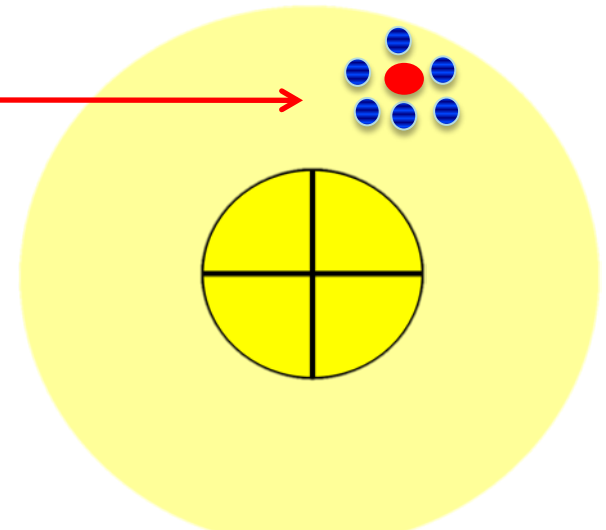
- **“PRECISION” IS A COMPUTED STATISTICAL QUANTITY TO THE *SOURCE* OF THE MEASUREMENT** - ALIGNMENT TO THE RTN OR PASSIVE MARK SHOWS PRECISION OF THE OBSERVATION (PER THE DATA COLLECTOR).
- **“ACCURACY” IS A COMPUTED STATISTICAL QUANTITY TO THE REALIZATION OF THE DATUM** - ALIGNMENT OF THE RTN OR PASSIVE MARK TO THE NSRS SHOWS ACCURACY (PER ESTABLISHED METHODOLOGY)



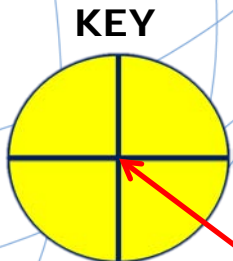
PRECISION vs. ACCURACY



ACCURATE – NOT PRECISE

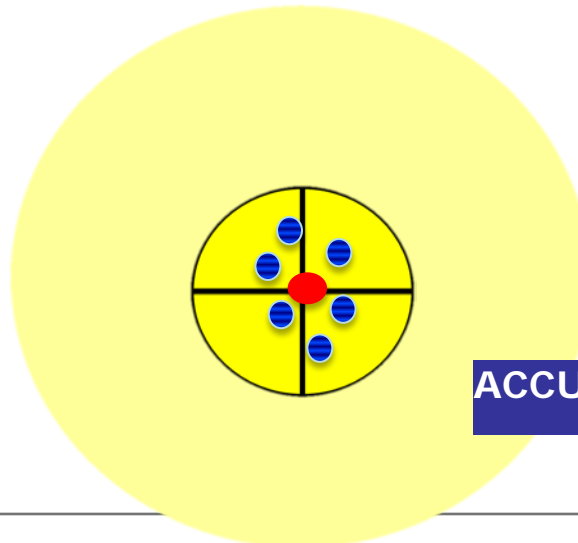


PRECISE – NOT ACCURATE



KEY

"TRUTH"



ACCURATE AND PRECISE

BEST METHODS FROM THE GUIDELINES: THE 7 "C's"

- CHECK EQUIPMENT
- COMMUNICATION
- CONDITIONS
- CONSTRAINTS(OR NOT)
- COORDINATES
- COLLECTION
- CONFIDENCE

THE CONTROL IS AT THE POLE

ACHIEVING ACCURATE, RELIABLE POSITIONS USING GNSS REAL TIME TECHNIQUES

FROM NGS SINGLE BASE GUIDELINES
CHAPTER 5 - FIELD PROCEDURES, AND
“USERS” CHAPTER OF RTN GUIDELINES:

RT = single base, either active or
passive

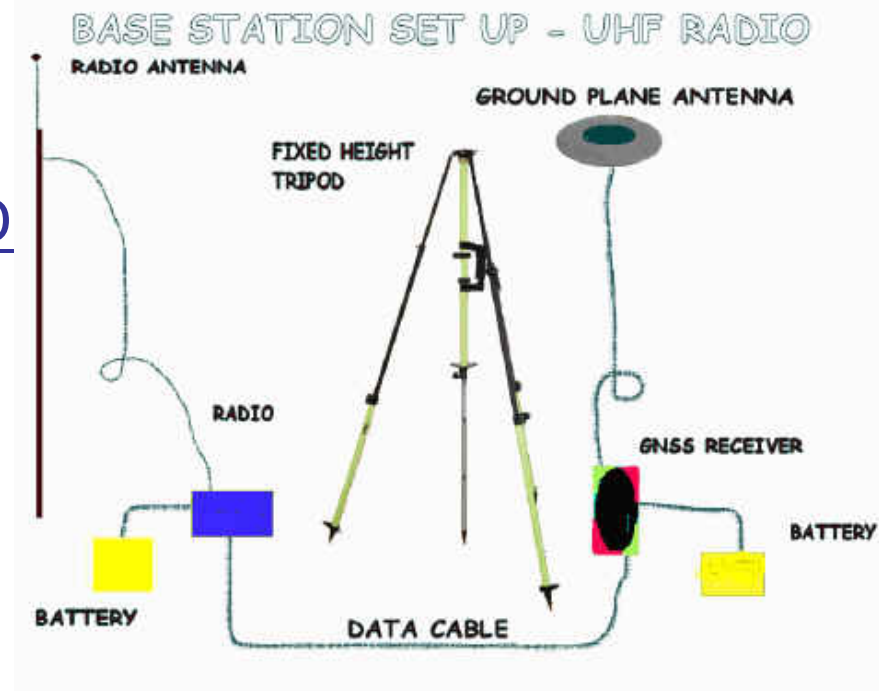
B = Both Single base and RTN

CHECK EQUIPMENT

- **B** BUBBLE- ADJUSTED?
- **RT** BATTERY- BASE FULLY CHARGED 12V?
- **B** BATTERY – ROVER SPARES?
- **RT** USE PROPER RADIO CABLE (REDUCE SIGNAL LOSS)
- **RT** RADIO MAST HIGH AS POSSIBLE? (5' = 5 MILES, 20' = 11 MILES, DOUBLE HEIGHT=40% RANGE INCREASE). LOW LOSS CABLE FOR >25'.
- **RT** DIPOLE (DIRECTIONAL) ANTENNA NEEDED?
- **RT** REPEATER?
- **RT** CABLE CONNECTIONS SEATED AND TIGHT?
- **B** "FIXED HEIGHT" CHECKED?
- **RT** BASE SECURE?

COMMUNICATION

- RT UHF FREQUENCY CLEAR?
- B CDMA/CELL - STATIC IP FOR COMMS?
- B CONSTANT COMMS WHILE LOCATING
- RT BATTERY STRENGTH OK?
- B CELL COVERAGE?
- B KEEP FIRMWARE UPDATED



CONDITIONS

- RT WEATHER CONSISTENT BASE & ROVER?
- B CHECK SPACE WEATHER?
- B CHECK PDOP/SATS FOR THE DAY?
- RT OPEN SKY AT BASE?
- RT MULTIPATH AT BASE?
- B MULTIPATH AT ROVER?
- B USE BIPOD?

Dilution Of Precision - DOP

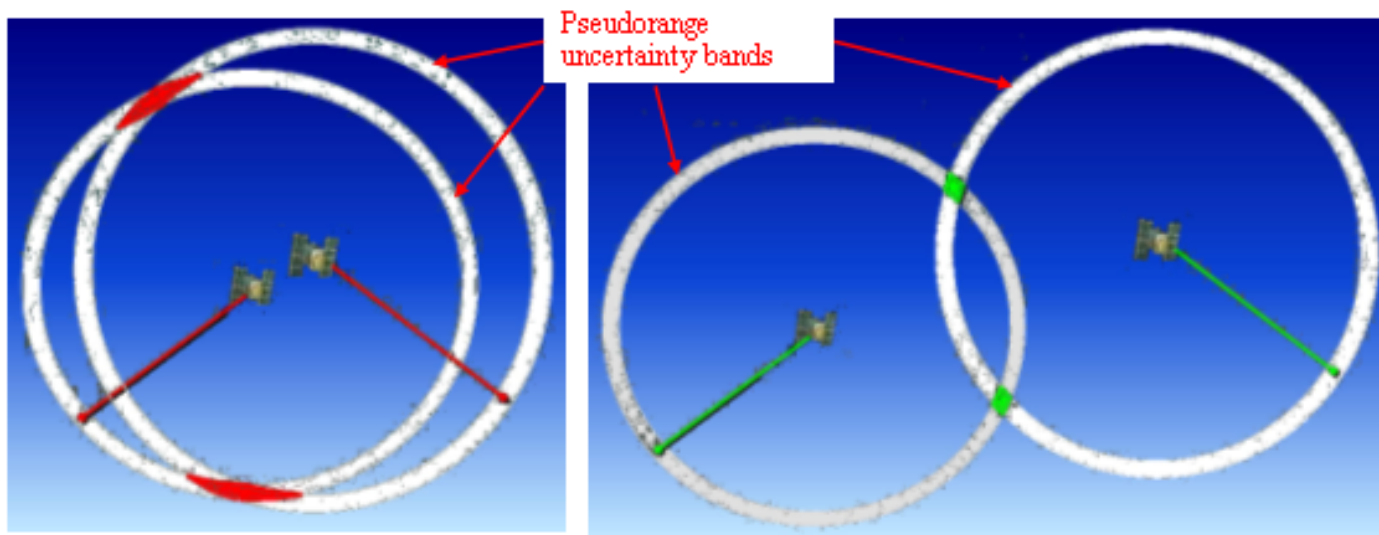


Diagram II-2

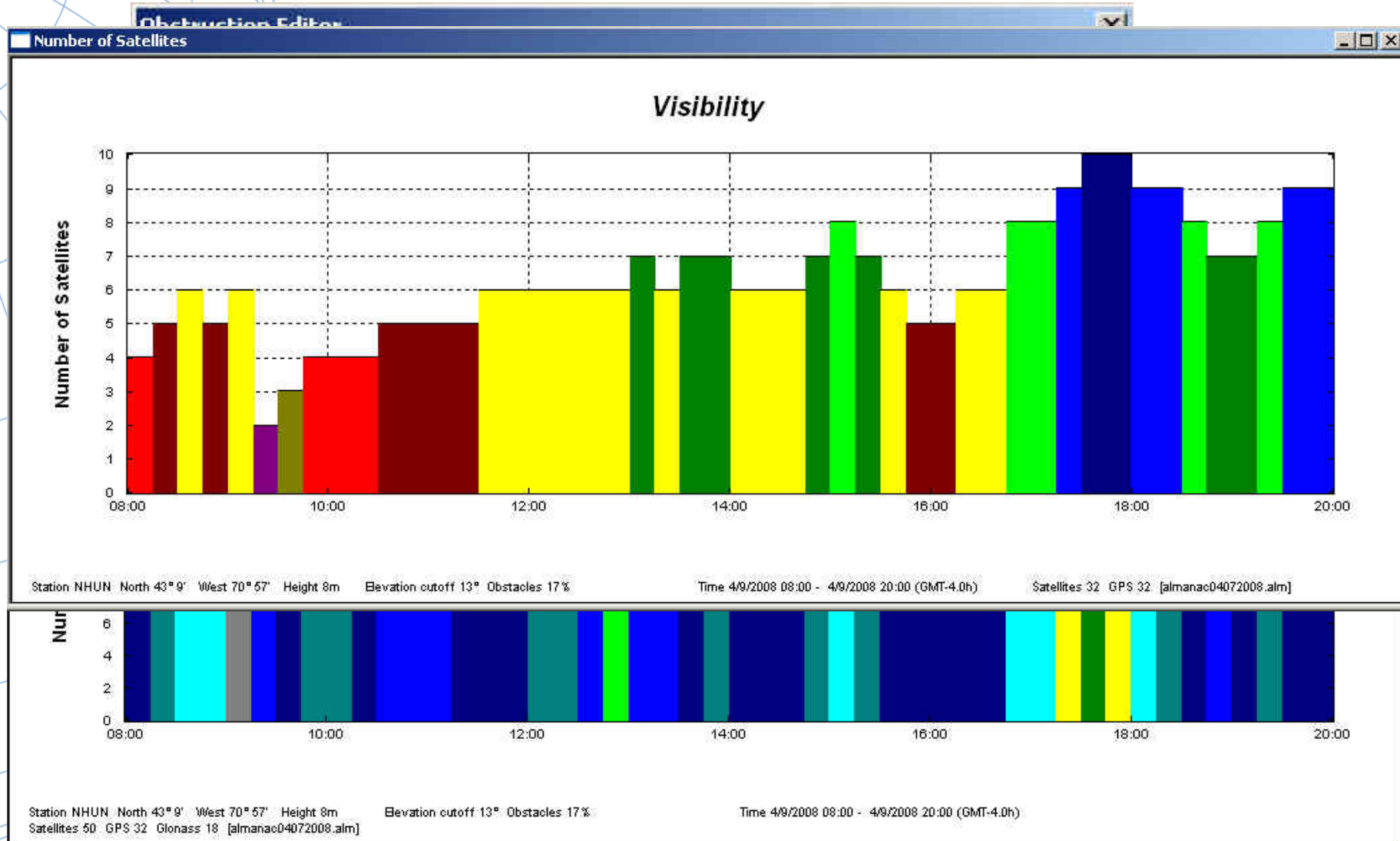
Diagram II-3

High PDOP - Satellites Close Together

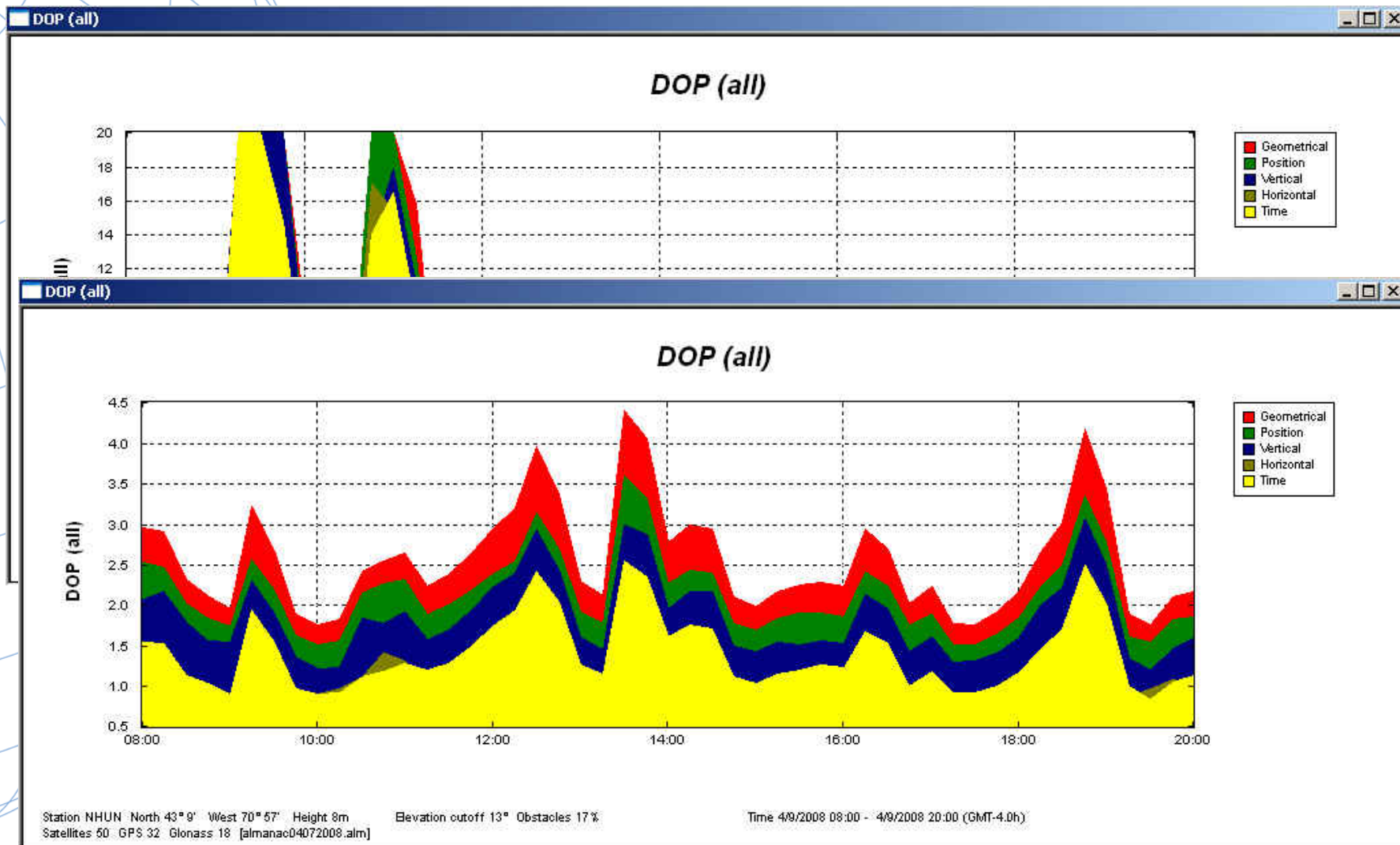
Low PDOP - Satellites Spread.

Note the difference in area of the intersections. In a three Dimensional sense with multiple satellites, it would be reflected in the difference of hyperbolic intersections displayed in polyhedron volumes. Mathematically, the lowest possible volume polyhedron formed by the signal intersections would have the lowest PDOP.

NHUN – SATELLITES/ WITH OBSTRUCTIONS



NHUN – DOP / WITH OBSTRUCTIONS



GPS AND GLN

Table 1 Comparison of GLONASS and GPS Characteristics

Parameter	Detail	GLONASS	GPS
Satellites	Number of satellites	21 + 3 spares ^a	21 + 3 spares ^a
	Number of orbital planes	3	6
	Orbital plane inclination (degrees)	64.8	55
	Orbital radius (kilometers)	25 510	26 560
Signals	Fundamental clock frequency (MHz)	5.0	10.23
	Signal separation technique ^b	FDMA	CDMA
	Carrier frequencies (MHz) L1	1598.0625 - 1609.3125 ^c	1575.42
	L2	1242.9375 - 1251.6875	1227.6
	Code clock rate (MHz) C/A	0.511	1.023
	P	5.11	10.23
	Code length (chips) C/A	511	1 023

DUAL CONSTELLATION RT POSSIBILITIES:

GPS \geq 5, GLN = 0

GPS = 4, GLN = 2

GPS = 3, GLN = 3

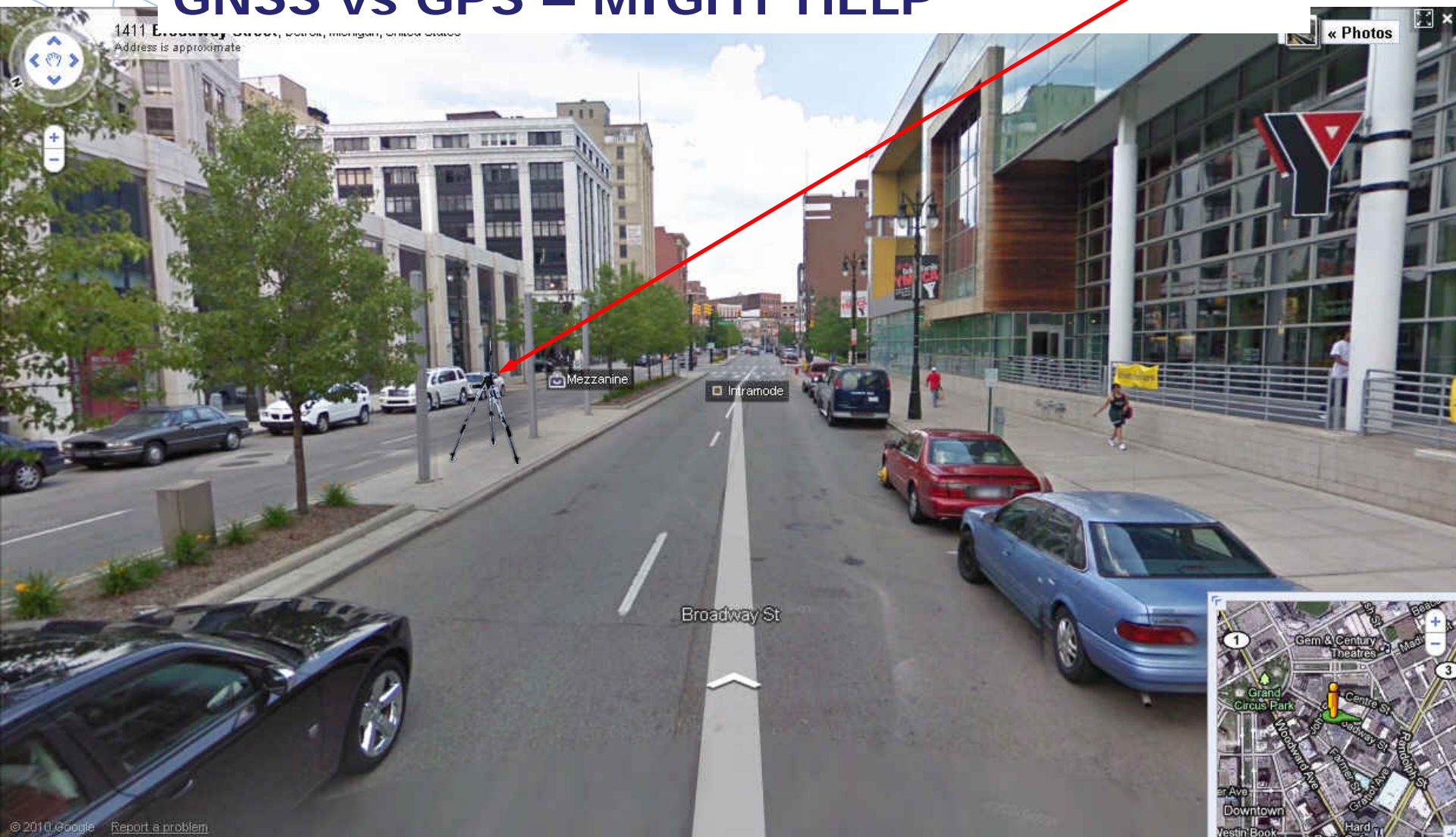
GPS = 2, GLN = 4

(Can't initialize with only GLN Sats.)



URBAN CANYONS

GNSS vs GPS – MIGHT HELP



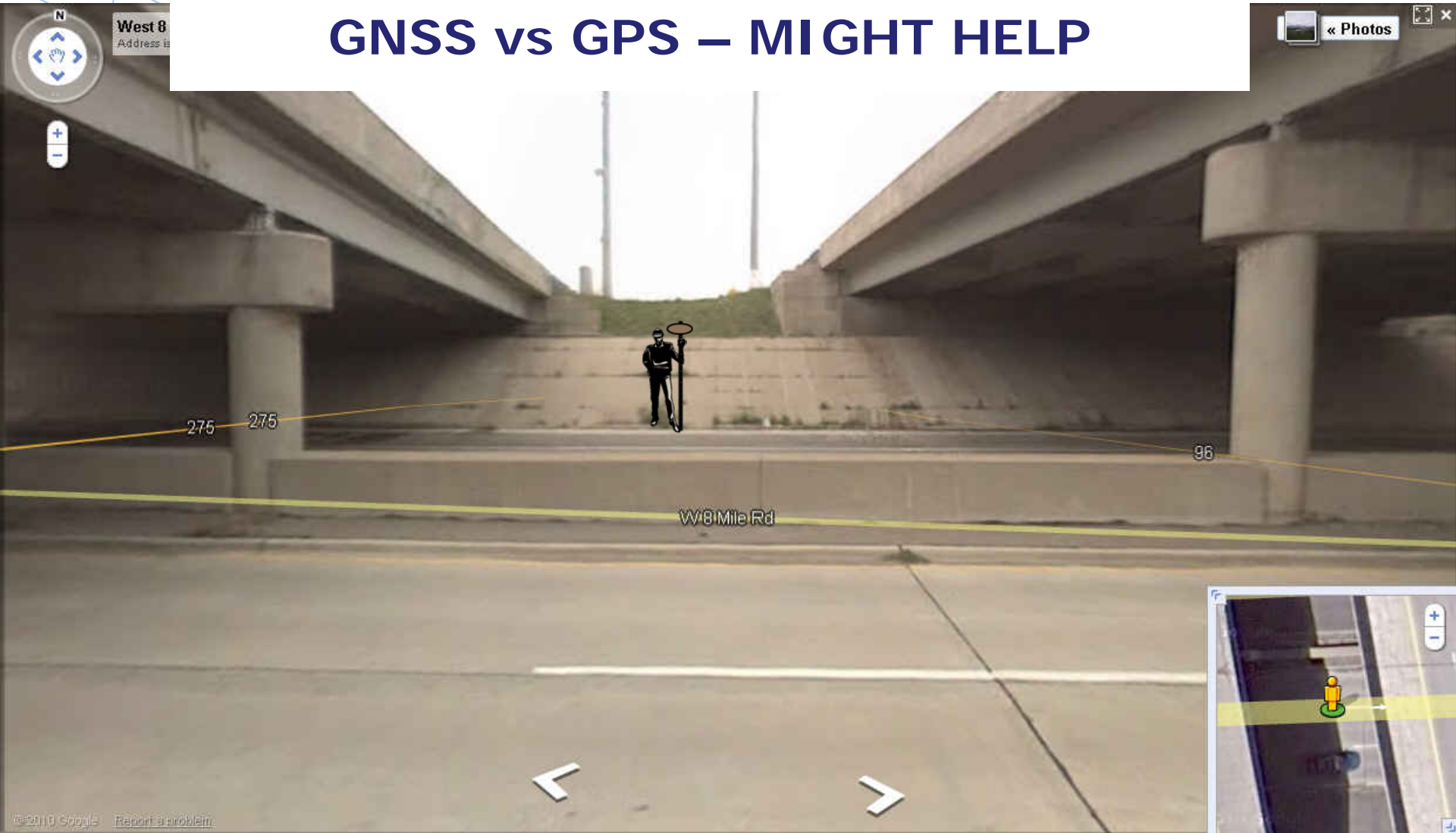
HIGHWAY CORRIDORS

GNSS vs GPS – MIGHT HELP



ROUTE OBSTRUCTIONS

GNSS vs GPS – MIGHT HELP



COORDINATES

- **B** TRUSTED SOURCE?
- **B** WHAT DATUM/EPOCH ARE NEEDED?
- **RT GIGO★**
- **B** ALWAYS CHECK KNOWN POINTS.
- **B** PRECISION VS. ACCURACY
- **B** GROUND/PROJECT VS. GRID/GEODETIC
- **B** GEOID MODEL QUALITY
- **B** LOG METADATA

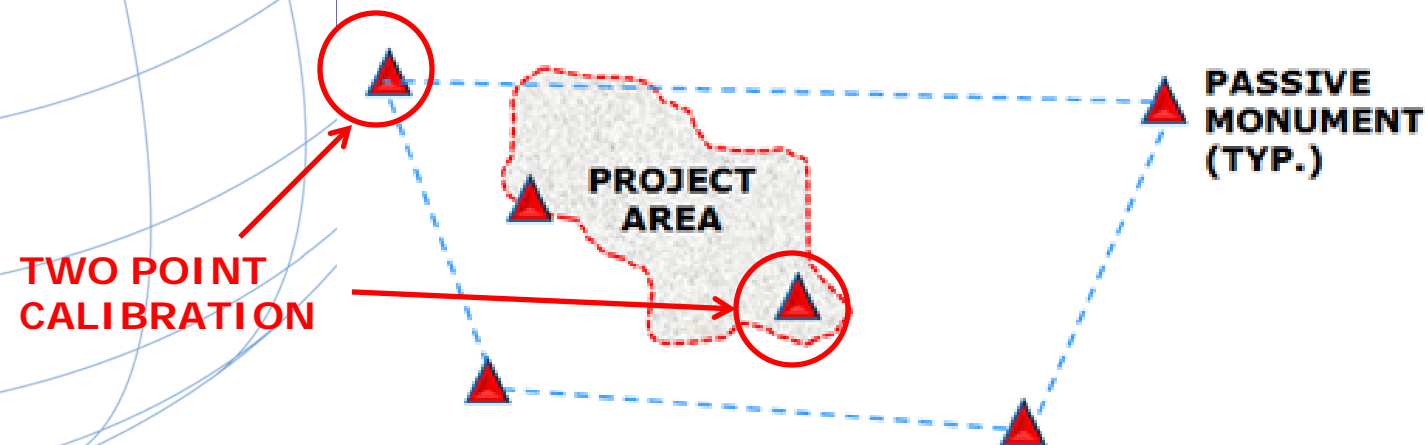
★ AUTONOMOUS LOCAL BASE STATION POSITION ARE OK IF CORRECT COORDINATES ARE INTRODUCED IN THE PROJECT FIRMWARE/SOFTWARE LATER

CONSTRAINTS (OR NOT)

- $B \geq 4 H \& V$, KNOWN & TRUSTED POINTS?
- B LOCALIZATION RESIDUALS-OUTLIERS?

FYI: GNSS CAN PROVIDE GOOD *RELATIVE* POSITIONS IN A PROJECT WHILE STILL NOT CHECKING TO KNOWN IN AN ABSOLUTE SENSE

- B SAME OFFICE & FIELD CALIBRATION



PASSIVE/ACTIVE

- NAD 83 REALIZED THROUGH NATIONAL CORS
- NAVD 88 REALIZED FROM PASSIVE MARKS
- NGSIDB HAS 1,000,000 PASSIVE MARKS
- PASSIVE MARKS IN STATES HAVE MANY CAMPAIGNS OVER MANY YEARS WITH MANY ACCURACIES IN MANY SEPARATE ADJUSTMENTS. RTN MAY NOT AGREE WITH THESE REQUIRING CONSTRAINTS TO THE MONUMENTS FOR PROJECT WORK.
- PASSIVE MARKS COORDINATES ARE A SNAPSHOT IN TIME AND CAN BE RELIED ON TO BE ACCURATE ONLY AT THE RECORDED OBSERVATION TIME.
- 2020+ = NEW GEOMETRIC DATUM / 1 CM GRAVIMETRIC GEOID, VELOCITIES ON A GEOCENTRIC DATUM
- THE METHOD OF CHOICE TO ACCESS THE NSRS IS ACTIVE MONUMENTATION (CORS, OPUS, RTN)



POSITIONAL ACCURACY

REPLACING DISTANCE CORRELATED ACCURACY

```

NE1027 *****
NE1027 CBN - This is a Cooperative Base Network Control Station.
NE1027 DESIGNATION - F 337
NE1027 PID - NE1027
NE1027 STATE/COUNTY- MI/WAYNE
NE1027 USGS QUAD - YPSILANTI EAST (1983)
NE1027
NE1027 *CURRENT SURVEY CONTROL
NE1027
NE1027* NAD 83 (2007) - 42 13 10.41682 (N) 083 30 40.50685 (W) ADJUSTED
NE1027* NAVD 88 - 212.637 (meters) 697.63 (feet) ADJUSTED
NE1027
NE1027 EPOCH DATE - 2002.00
NE1027 X - 534,616.161 (meters) COMP
NE1027 Y - -4,700,472.689 (meters) COMP
NE1027 Z - 4,263,815.954 (meters) COMP
NE1027 LAPLACE CORR- -1.96 (seconds) DEFLEC09
NE1027 ELLIP HEIGHT- 178.286 (meters) (06/10/07) ADJUSTED
NE1027 GEOID HEIGHT- -34.35 (meters) GEOID09
NE1027 DYNAMIC HT - 212.570 (meters) 697.41 (feet) COMP
NE1027
NE1027 SUPERSEDED SURVEY CONTROL

```

```

NE1027
NE1027
NE1027 ELLIP H (02/10/07) 178.277 (m) GP ( ) cm) -----
NE1027 ELLIP H (06/11/02) 178.311 (m) GP ( ) East Ellip
NE1027 NAD 83 (1994) - 42 13 10.41653 (N) 083 30 40.50656 (W) AD ( ) 0.31 0.94
NE1027 ELLIP H (09/20/95) 178.321 (m) GP ( ) 1 2 NAVD 88
NE1027 NAD 83 (1986) - 42 13 10.42375 (N) 083 30 40.53514 (W) AD ( ) 1 GRAV_OBS
NE1027 NAD 83 (1986) - 42 13 10.42214 (N) 083 30 40.53071 (W) AD ( ) 1
NE1027 NAVD 88 (09/30/94) 212.64 (m) 697.6 (f) LEVELING 3
NE1027 NGVD 29 (01/19/93) 212.768 (m) 698.06 (f) ADJUSTED 1 2 vations
NE1027

```

NE1027



Monumented Points Deterioration

Disturbed Geodetic Control
Coordinates/Elevations
Questionable!



Destroyed Geodetic Control
No Coordinates/Elevation

RT DERIVED ORTHO HEIGHTS - LOCALIZE OR NOT?

- PASSIVE MARKS ARE A SNAP SHOT OF WHEN THEY WERE LEVELED OR DERIVED FROM GPS
- IF YOU BUILD FROM A MONUMENTED BM AND THE DESIGN WAS DONE REFERENCED TO IT, IT IS "THE TRUTH", UNLESS IN GROSS ERROR.
- CONSTRAINING TO PASSIVE BMs IS A GOOD WAY TO NOT ONLY LOCK TO THE SURROUNDING PASSIVE MARKS, BUT ALSO TO EVALUATE HOW THE CONTROL FITS TOGETHER.
- HOW GOOD IS THE NGS **HYBRID GEOID MODEL** IN YOUR AREA? (SIDE NOTE: GEOID 09 IS THE CURRENT MODEL USED BY OPUS)

USING RTN ELLIPSOID HEIGHTS WITH THE HYBRID GEOID FOR ORTHO HEIGHTS

$$H_{88} = h_{83} - N_{09}$$

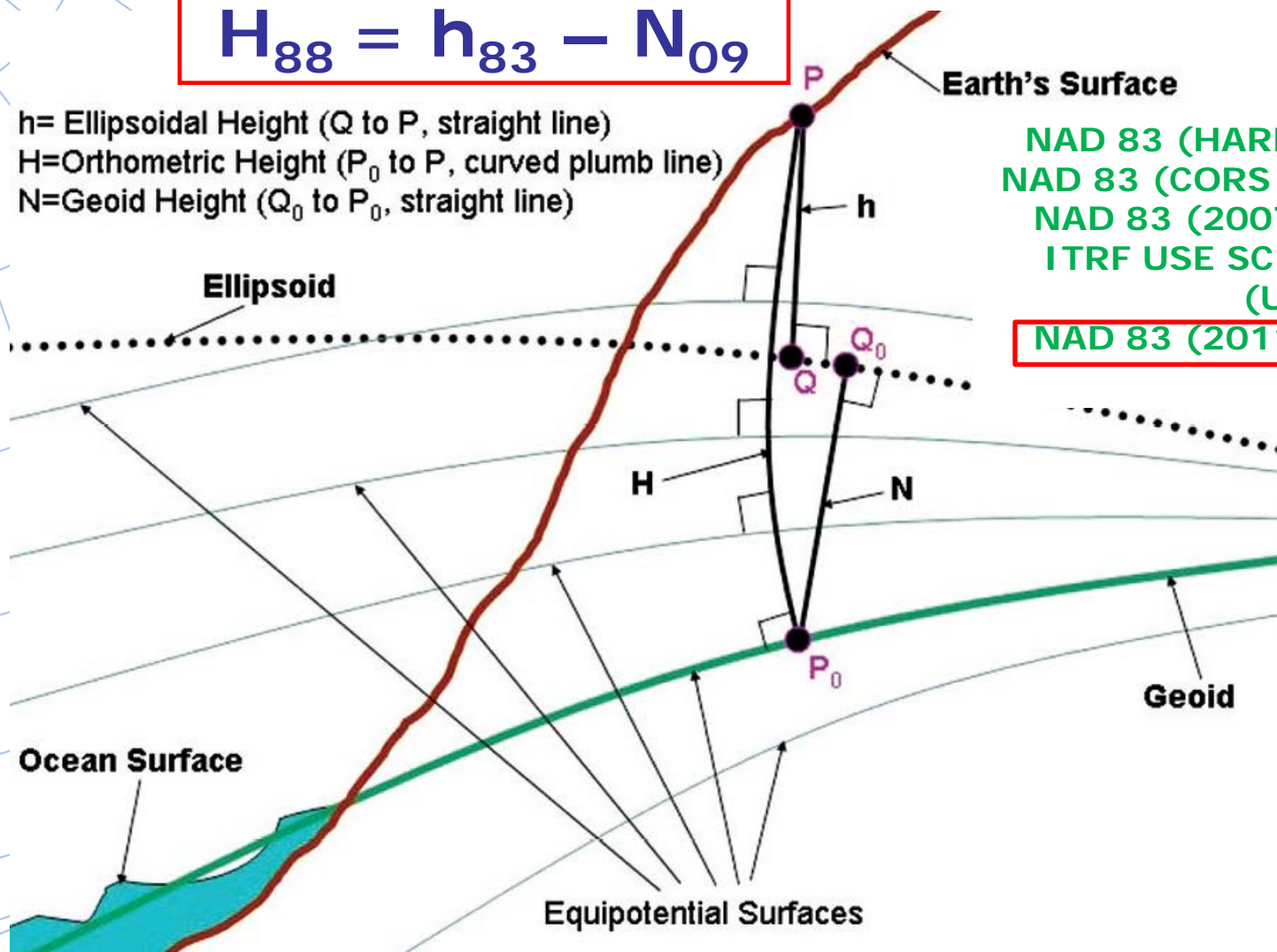
h = Ellipsoidal Height (Q to P, straight line)

H = Orthometric Height (P_0 to P, curved plumb line)

N = Geoid Height (Q_0 to P_0 , straight line)

NAD 83 (HARN) USE GEOID 03
NAD 83 (CORS 96) USE GEOID 09
NAD 83 (2007) USE GEOID 09
ITRF USE SCIENTIFIC GEOID
(USGG)

NAD 83 (2011) USE GEOID 12



CALIBRATIONS/VERTICAL LOCALIZATIONS

Residual Differences Between GPS A

Horizontal
Vertical
Three-dimensional

GPS point	
Point	
Latitude	39°09'42.25945"N
Longitude	76°39'26.89353"W
Height	

Point	
Latitude	39°09'42.25945"N
Longitude	76°39'26.89353"W
Height	

Point	
Latitude	39°09'42.25945"N
Longitude	76°39'26.89353"W
Height	

Point	HARWOODG
Latitude	39°11'59.51096"N
Longitude	76°44'16.00127"W
Height	83.693sft

GPS Site Calibration - Point List

Points:

Name	Value
-GPS Point	FRIENDG
-Latitude	39°09'42.25945"N
-Longitude	76°39'26.89353"W
-Height	-8.716sft
-Grid Point	FRIEND
-Northing	544668.360sft
-Easting	1409452.970sft
-Elevation	98.040sft
-Type	Horz and Vert
-GPS Point	LR3G
-Grid Point	LR3
-Type	Horz and Vert

Statistics:

Horizontal adjustment scale factor: 0.99996488

Max vertical adjustment inclination: 33.317ppm

Max horizontal residual: 0.035sft

Max vertical residual: 0.006sft

Buttons: OK, Cancel, Insert, Delete

Vertical error	0.004sft
3D error	0.035sft
Northing	558479.031sft
Easting	1386642.076sft
Elevation	189.562sft
Horizontal error	0.027sft
Vertical error	0.002sft
3D error	0.027sft

Mean Square error	Point
0.009	GIS86G
0.001	LR3G
0.009	GIS86G

Control point	
Point	FRIEND
Northing	544668.360sft
Easting	1409452.970sft
Elevation	98.040sft
Type	Horz and Vert
Point quality	Control quality

Point	LR3
Northing	555685.700sft
Easting	1408208.310sft
Elevation	140.092sft
Type	Horz and Vert
Point quality	Control quality

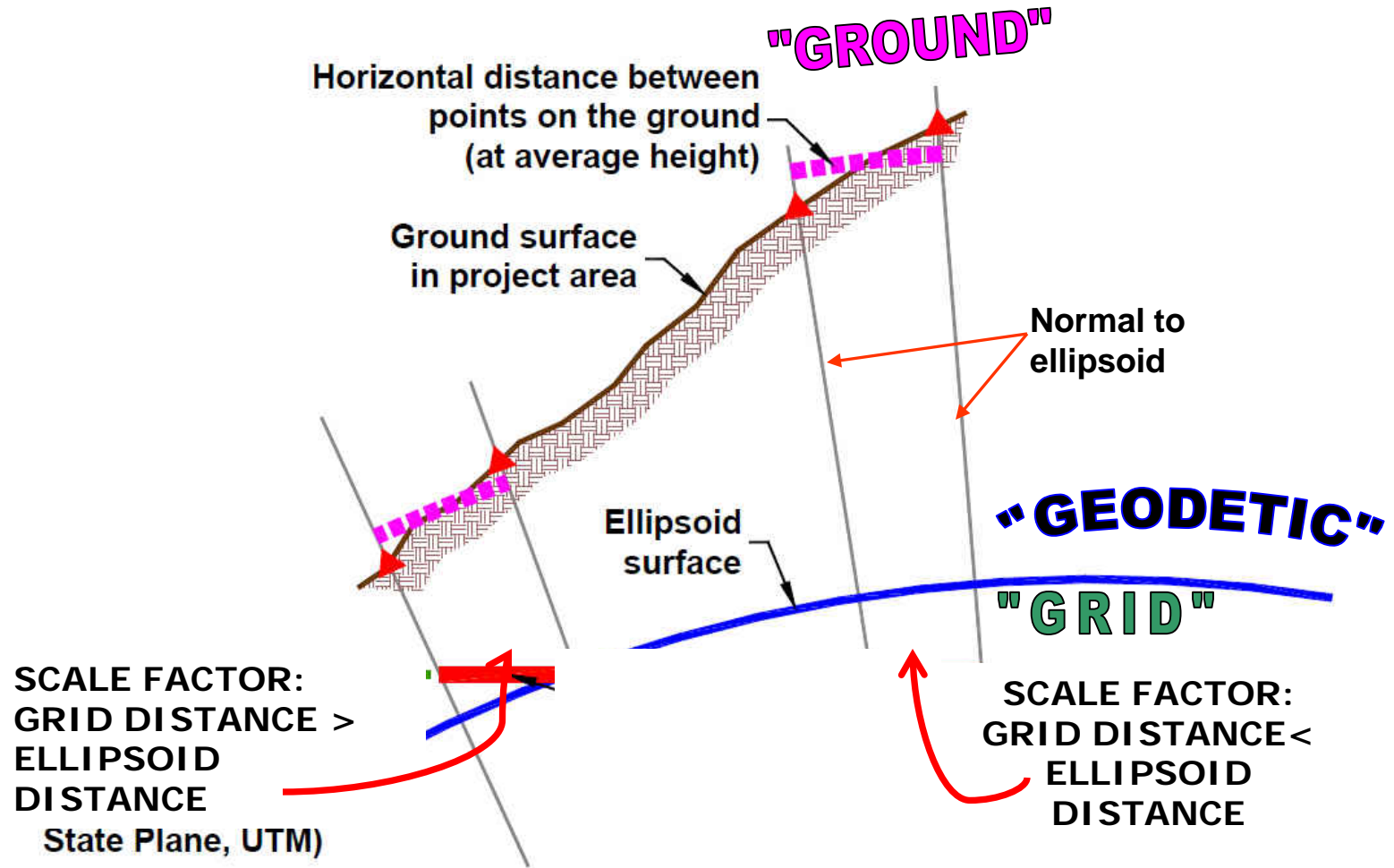
Point	GIS86
Northing	566928.280sft
Easting	1397313.260sft
Elevation	57.130sft
Type	Horz and Vert
Point quality	Control quality

Point	HARWOOD
Northing	558479.010sft
Easting	1386642.060sft
Elevation	189.560sft
Type	Horz and Vert
Point quality	Control quality



GRID/GROUND ISSUE

Linear distortion due to ground height above ellipsoid



PROJECT SURFACE VS. GRID PROJECTION SURFACE IS YOUR DATA COLLECTOR CONFIGURED TO HANDLE THE TRANSFORMATION?

- FEATURES AND WORK ARE REFERENCED TO THE GROUND
- CONTROL MONUMENTATION IS USUALLY REFERENCED TO THE GRID
- THERE ARE DIFFERENT WAYS TO RESOLVE THIS:
 1. MODIFIED SPC
 2. LDP
 3. LOCALIZATION TO PASSIVE MONUMENTATION
 4. ASSUMED (TANGENT PLANE)

**RTN CAN ENCOMPASS LARGE REGIONS
COMPOSED OF MANY STATES!**



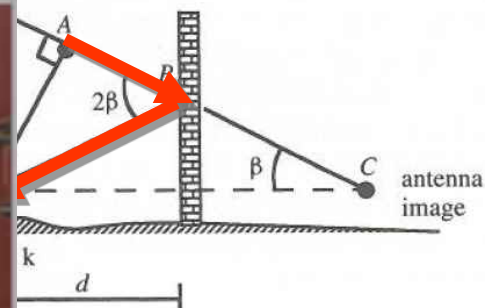
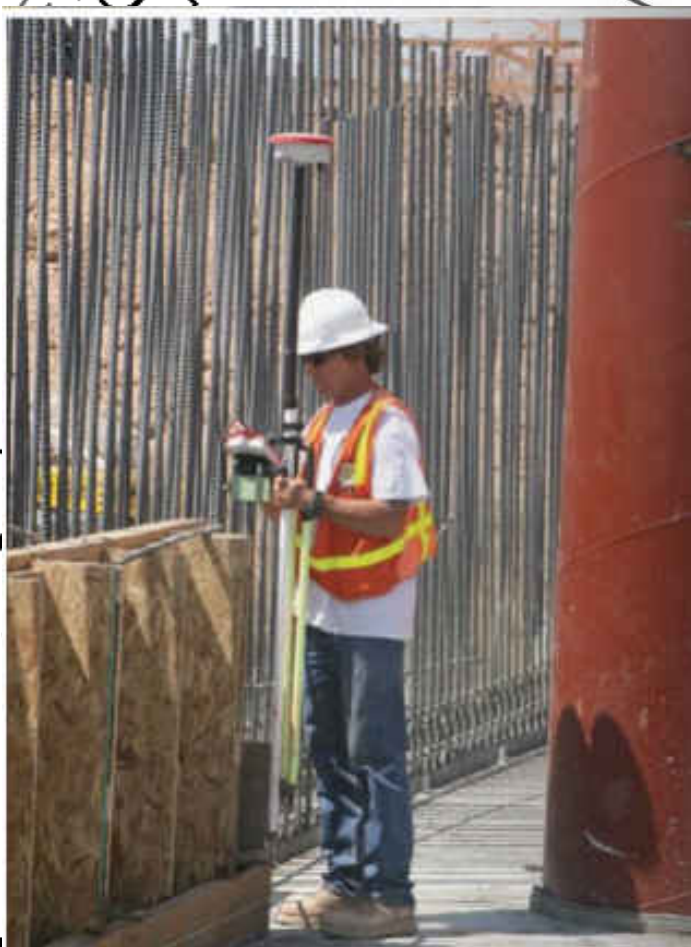
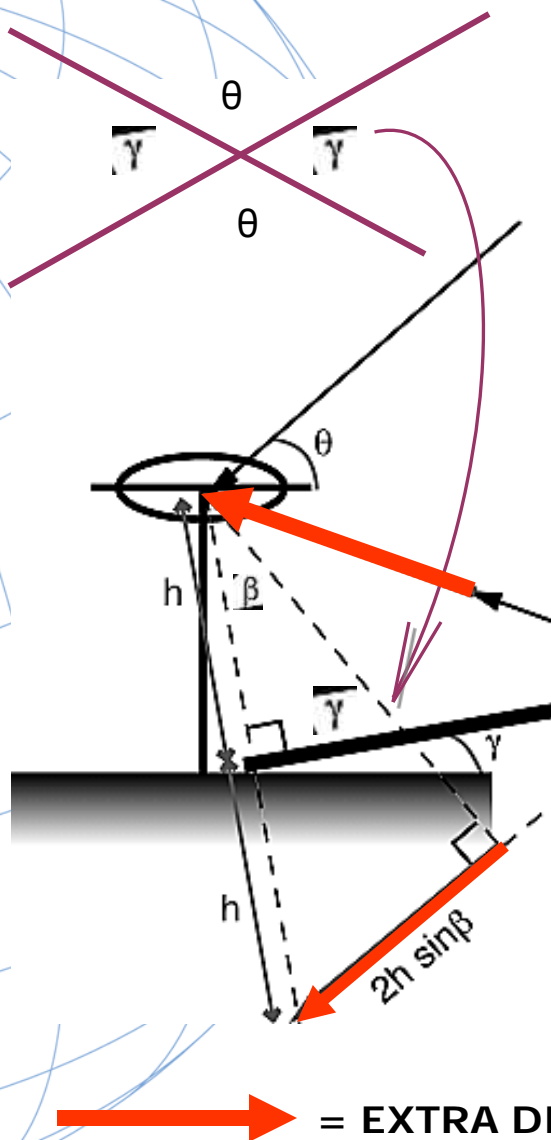
COLLECTION

- **B** CHECK ON KNOWN POINTS!
- **B** SET ELEVATION MASK
- **B** ANTENNA TYPES ENTERED OK?
- **B** SET COVARIANCE MATRICES ON (IF NECESSARY).
- **B** RMS SHOWN IS TYPICALLY 68% CONFIDENCE (BRAND DEPENDENT)
- **B** H & V PRECISION SHOWN IS TYPICALLY 68% CONFIDENCE
- **B** TIME ON POINT? QA/QC OF INTEGER FIX
- **B** MULTIPATH? DISCRETE/DIFFUSE
- **B** BUBBLE LEVELED?
- **B** PDOP?
- **B** FIXED SOLUTION?
- **B** USE BIPOD?
- **B** COMMS CONTINUOUS DURING LOCATION?
- **B** BLUNDER CHECK LOCATION ON IMPORTANT POINTS.

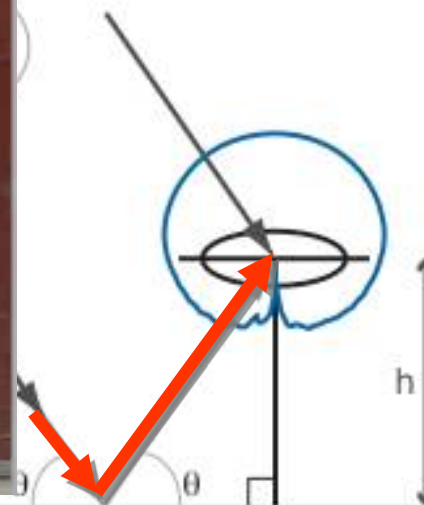
COLLECTION ENVIRONMENT

- **WHAT ABOUT TREE CANOPY?**
- **WHAT ABOUT HIGH POWER TRANSMISSION LINES?**
- **WHAT ABOUT SNOW AND RAIN?**
- **WHAT ABOUT KINEMATIC PP WHERE COMMUNICATION IS LOST FOR A SHORT TIME?**
- **WHAT ABOUT LASER RANGE FINDERS?**
- **WHAT ABOUT INCLINOMETERS?**

MULTIPATH



reflection on a vertical planar plane.



MULTIPATH = NOISE SPECULAR(DISCRETE) & DIFFUSE

INSIDE GNSS

NOVEMBER-DECEMBER
2008

"MULTIATH-
MITIGATION
TECHNIQUES USING
MAXIMUM-LIKELIHOOD
PRINCIPLE"

MOHAMED SAHMOUDI
AND

RENE JR. LANDRY

WWW.INSIDEGNSS.COM

SOURCES:

TOWERS

BUILDINGS

WATER

VEHICLES

TREE CANOPY

STRUCTURES



NOTE:
Newer antennas
have much better
multipath
mitigation/rejection
than legacy
antennas

$$\varphi_k^p(t) = \frac{f}{c} \rho_k^p(t) - f dt_k(t) + f dt^p(t) + N_k^p - I_{k,\varphi}^p(t) + \frac{f}{c} T_k^p(t) + d_{k,\varphi}(t) + d_{k,\varphi}^p(t) + d_{\varphi}^p(t) + s_{\varphi}$$

CONFIDENCE

- **B CHECK** KNOWN BEFORE, DURING, AFTER SESSION.
- **B NECESSARY REDUNDANCY?** (U. of
CAN'T INITIALIZE? BAD CHECKS? PLENTY OF SATS? TRY:
 - **TURN OFF GLONASS IF YOU HAVE ≥ 6 COMMON GPS SATS**
 - **REINITIALIZE**
 - **CHECK FOR "NOISY" SATS IN DATA COLLECTOR**
 - **LOOK FOR MULTIPATH NEARBY**
- **B BE AWARE OF POTENTIAL INTERFERENCE (E.G., HIGH TENSION TOWER LINES)**

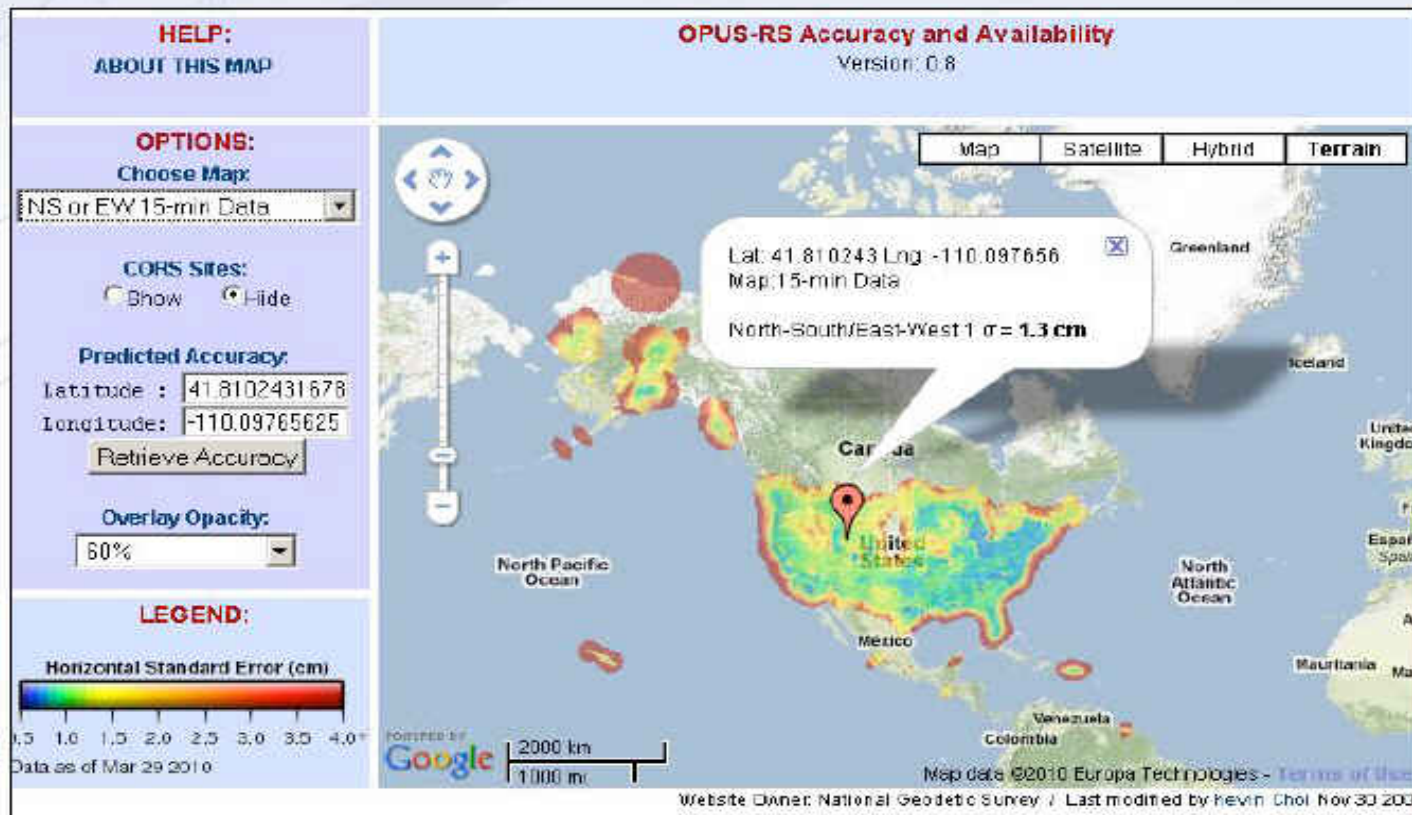
BLUNDER/MISMATCH CHECKING

NOAA's National Geodetic Survey Positioning America for the Future

www.ngs.noaa.gov

How Good Can I Do With OPUS-RS?

The OPUS-RS Accuracy and Availability Tool.

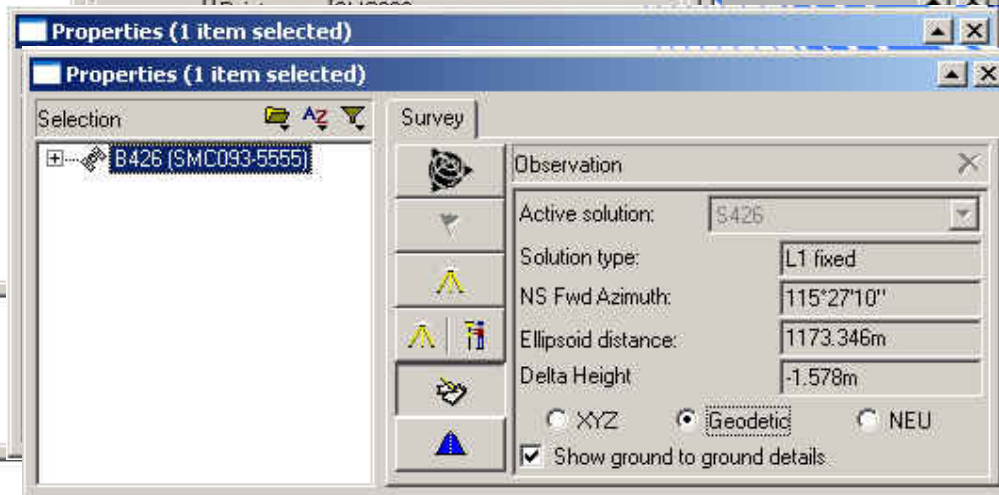
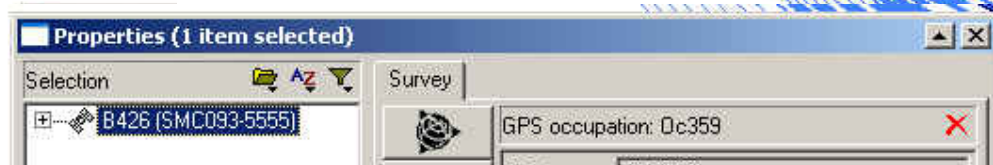
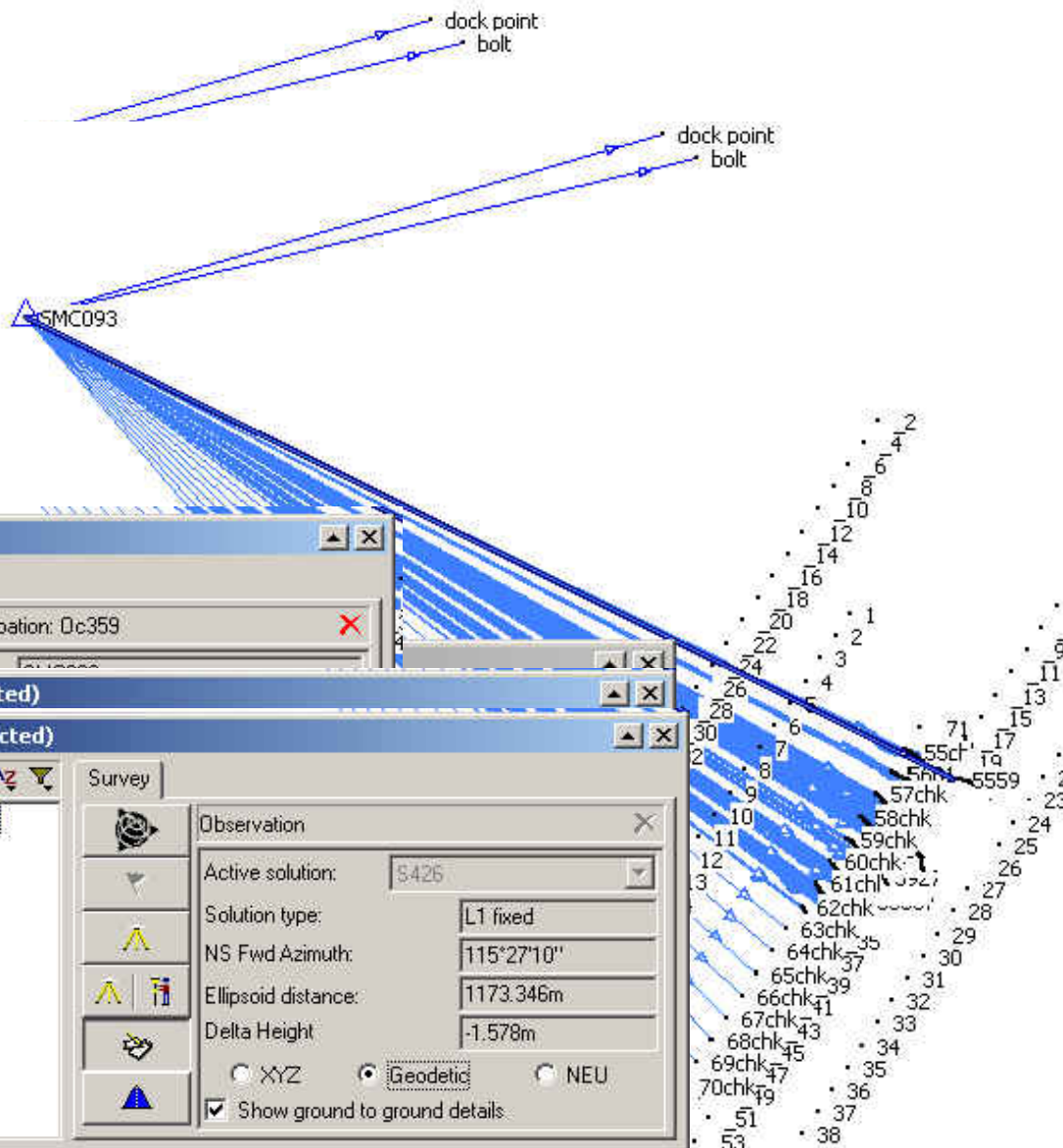


http://www.ngs.noaa.gov/OPUS/Plots/Gmap/OPUSRS_sigmap.shtml

FURTHER CHECKS IN THE OFFICE

- Antenna heights (height blunders are unacceptable and can even produce horizontal error - Meyer, et.al, 2005).
- Antenna types
- RMS values
- Redundant observations
- Horizontal & vertical precision
- PDOP
- Base station coordinates
- Number of satellites
- Calibration (if any) residuals

VECTOR & EQUIPMENT REVIEW



METADATA !

BESIDES ATTRIBUTE FIELDS, THE RT PRACTICIONER MUST KEEP RECORDS OF ITEMS NOT RECORDED IN THE FIELD, FOR INSTANCE:

- ✓ **WHAT IS THE SOURCE OF THE DATA?**
- ✓ **WHAT WAS THE DATUM/ADJUSTMENT/EPOCH?**
- ✓ **WHAT WERE THE FIELD CONDITIONS?**
- ✓ **WHAT EQUIPMENT WAS USED, ESPECIALLY- WHAT ANTENNA?**
- ✓ **WAS COMMUNICATION SOLID?**
- ✓ **WHAT FIRMWARE WAS IN THE RECEIVER & COLLECTOR?**
- ✓ **WERE ANY GUIDELINES USED FOR COLLECTION?**
- ✓ **WHAT REDUNDANCY, IF ANY, WAS USED?**
- ✓ **WERE ANY PASSIVE MARKS CONSTRAINED?**

(GOOD IDEA TO CREATE A TABULAR CHECK LIST FORM)



KNOW YOUR METADATA

**ALL THESE COME
INTO PLAY TO
ENABLE THE
STRUCTURE TO
CLEAR THE
BRIDGE!**



- LMSL
- NAD 83
- NAVD 88
- BATHYMETRY
- CHART DATUM
- BRIDGE DYNAMICS
- BRIDGE DIMENSIONS
- SHIP SQUAT
- SHIP DIMENSIONS



QUICK FIELD SUMMARY:

- Set the base at a wide open site
- Set rover elevation mask between 10° & 15°
- The more satellites the better
- The lower the PDOP the better
- The more redundancy the better
- Beware multipath
- Beware long initialization times
- Beware antenna height blunders
- Survey with “fixed” solutions only
- Always check known points before, during and after new location sessions
- Keep equipment adjusted for highest accuracy
- Communication should be continuous while locating a point
- Precision displayed in the data collector can be at the 68 percent level (or 1σ), which is only about half the error spread to get 95 percent confidence
- Have back up batteries & cables
- RT doesn't like tree canopy or tall buildings



FOUR CARDINAL RULES FOR RT POSITIONING

- COMMUNICATIONS: THE KEY TO SUCCESS
- CHECK SHOT: FIRST BEFORE NEW WORK
- REDUNDANCY: FOR CONFIDENCE
- MULTIPATH: AVOID UNSUITABLE CONDITIONS

CRADLE TO GRAVE GNSS!



GPS Helps Track Babies in Nurseries

Hospitals all over the world are starting to use GPS to track newborns in their nurseries as a security measure.



Instead of looking for a traditional tombstone to mark the final resting place of a loved one, friends and relatives will be able to find the location of the deceased using a GPS device or mobile phone.

"The park will look very natural, just grass and trees. There will be no headstones and instead people will be buried in the park and a GPS locator placed in the coffin," Michael McMahon chief executive of the [Catholic Cemeteries Board](#) told ABC News.